

From last time

2.3.4.1 Continuum weak statement (WK)

The weak statement for elastodynamics and the boundary conditions are:

Find $u \in V = \{v \in C^1(D^t) \mid \forall x \in \partial D_u^t \ v(x) = \bar{u}(x)\}$, such that, (138a)

$\forall w \in W = \{v \in C^1(D^t) \mid \forall x \in \partial D_u^t \ v(x) = 0\}$, $\forall t \in I^t$ (138b)

$\int_D [\rho w \cdot \ddot{u} + \alpha w \cdot \dot{u} + \epsilon(w) : \sigma(u)] \, dv = \int_D w \cdot \rho b \, dv + \int_{\partial D_f} w \cdot \bar{t} \, ds$ (138c)

- Both V and W have the same regularity ($C^m(D)$): $m = M/2$, $M = 2$ is the order of the differential equation.
- The less demanding regularity conditions for the solution compared to the weighted residual statement ($C^M(D) \rightarrow C^m(D)$) takes us to the same function space needed for the balance law (highest derivative is for $\sigma(u) = C_{ijkl} u_{k,l}$ is 1).
- Both V and W exactly enforce the essential boundary conditions, with the difference that W satisfies the homogeneous version.

Discretize the problem

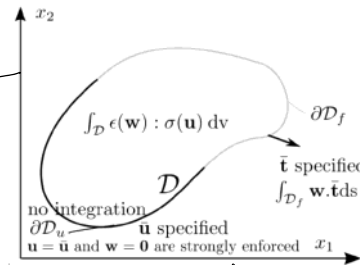
$u^h(x,t) = \sum_{i=1}^{n_f} a_i^f(t) \phi_i(x) \rightarrow u^h(x,t)$

$\forall x \in \partial D_0 \ \phi_i(x) = 0 \quad u^h(x,t) = \bar{u}(x,t)$

$\forall x \in \partial D_u \ u^h(x,t) = \bar{u}(x,t)$

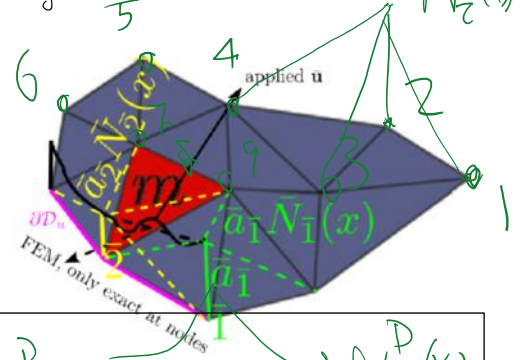
we use shape functions for ϕ 's

$u_p(x,t) = \sum_{i=1}^{n_p} a_i^p(t) N_i^p(x)$



how we satisfy essential BC

$n_f = 9$
 $n_p = 3$



unknown known

$u = \sum_{i=1}^{n_f} a_i^f(t) N_i^f(x) \rightarrow \sum_{i=1}^{n_p} a_i^p(t) N_i^p(x)$

$= N a$, $N = [N^f | N^p] = [N_1^f \dots N_{n_f}^f \mid N_1^p \dots N_{n_p}^p](x)$

$a = \begin{bmatrix} a^f \\ a^p \end{bmatrix}$ = prescribed known

- Discrete solution function can be written as (cf. (139), (140), (141)),

$$\mathbf{u}^h(\mathbf{x}, t) = \mathbf{u}^{fh} + \mathbf{u}^{ph}(\mathbf{x}, t) \quad (142a)$$

$$= \sum_{i=1}^{n_f} a_i^f(t) \mathbf{N}_i^f(\mathbf{x}) + \sum_{i=1}^{n_p} a_i^p(t) \mathbf{N}_i^p(\mathbf{x}) \quad (142b)$$

$$= \mathbf{N} \cdot \mathbf{a} \quad (142c)$$

where

$$\mathbf{N} = [\mathbf{N}^f \quad \mathbf{N}^p] = [\mathbf{N}_1^f \quad \dots \quad \mathbf{N}_{n_f}^f \quad | \quad \mathbf{N}_1^p \quad \dots \quad \mathbf{N}_{n_p}^p] \quad 3 \times n \text{ array} \quad (143a)$$

$$\mathbf{a} = \begin{bmatrix} \mathbf{a}^f \\ \mathbf{a}^p \end{bmatrix} = \begin{bmatrix} a_1^f \\ \vdots \\ a_{n_f}^f \\ \hline a_1^p \\ \vdots \\ a_{n_p}^p \end{bmatrix} \quad n \times 1 \text{ array} \quad (143b)$$

$$\mathbf{u}^h(\mathbf{x}, t) \quad 3 \times 1 \text{ array} \quad (143c)$$

$$n = n_f + n_p \quad \text{total number of dofs} \quad (143d)$$

Notes:

- Unknown quantities, \mathbf{a}^f are shown in this color. There are n_f unknowns in \mathbf{a} .

2.3.4.1 Continuum weak statement (WK)

The weak statement for elastodynamics and the boundary conditions are:

$$\text{Find } \mathbf{u} \in \mathcal{V} = \{v \in C^1(\mathcal{D}^t) \mid \forall \mathbf{x} \in \partial \mathcal{D}_u^t, v(\mathbf{x}) = \bar{\mathbf{u}}(\mathbf{x})\}, \text{ such that,} \quad (138a)$$

$$\forall \mathbf{w} \in \mathcal{W} = \{v \in C^1(\mathcal{D}^t) \mid \forall \mathbf{x} \in \partial \mathcal{D}_u^t, v(\mathbf{x}) = \mathbf{0}\}, \forall t \in \mathcal{I}^t \quad (138b)$$

$$\int_{\mathcal{D}} [\rho \mathbf{w}(\ddot{\mathbf{u}}) + \alpha \mathbf{w}(\dot{\mathbf{u}}) + \epsilon(\mathbf{w}) : \sigma(\mathbf{u})] dv = \int_{\mathcal{D}} \mathbf{w} \cdot \rho \mathbf{b} dv + \int_{\partial \mathcal{D}_f} \mathbf{w} \cdot \bar{\mathbf{t}} ds \quad (138c)$$

$$\mathbf{u} = \mathbf{N} \mathbf{a} \quad \dot{\mathbf{u}} = \mathbf{N} \dot{\mathbf{a}} \quad \ddot{\mathbf{u}} = \mathbf{N} \ddot{\mathbf{a}}$$

$$\mathbf{w} = ? \quad \mathbf{N}^t \quad \mathbf{N}_f^t \quad \mathbf{N}_p^t$$

$$\mathbf{N} = [\mathbf{N}_f \quad | \quad \mathbf{N}_p]$$

Need to form $\epsilon(\mathbf{w})$ & $\sigma(\mathbf{u})$

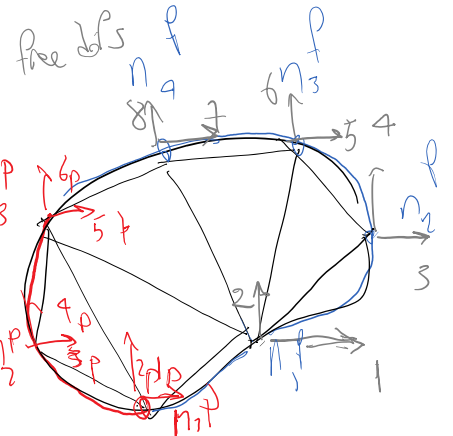
$$\sigma(\mathbf{u}) = \mathbb{C} \epsilon(\mathbf{u})$$

↳ st, fress

$$n_f = 8$$

$$n_p = 6$$

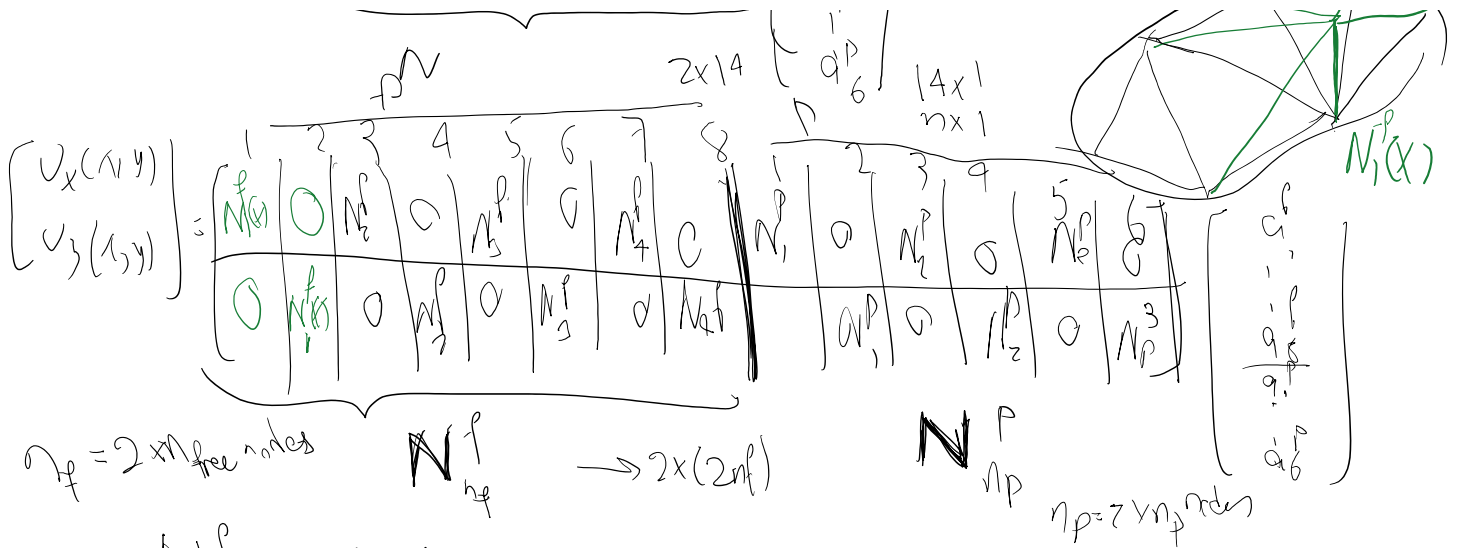
$$n = 14$$



Vector equation: $\mathbf{u} = \mathbf{N} \mathbf{a}$

$$\vec{\mathbf{u}}(x,y) = \begin{bmatrix} u_x(x,y) \\ v_y(x,y) \end{bmatrix} = \begin{bmatrix} \mathbf{N}_f & \mathbf{N}_p \end{bmatrix} \begin{bmatrix} \mathbf{a}_f \\ \mathbf{a}_p \end{bmatrix}$$

2×14



$$N_f^p = [N_1^p | N_2^p | N_3^p | N_4^p] \quad 1 \times 16$$

Voigt notation for strain & stress
2D

$$E = \begin{bmatrix} \epsilon_{11} & \epsilon_{12} \\ \epsilon_{21} & \epsilon_{22} \end{bmatrix} = \begin{bmatrix} u_{1,1} & \frac{1}{2}(u_{1,2} + u_{2,1}) \\ & u_{2,2} \end{bmatrix}$$



$$\sigma = C_{2 \times 2 \times 2 \times 2} E_{2 \times 2} = \begin{bmatrix} \sigma_{11} & \sigma_{12} \\ \sigma_{21} & \sigma_{22} \end{bmatrix}$$

sym

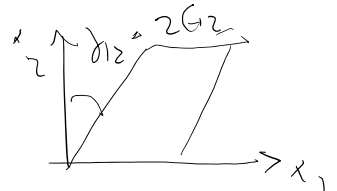
general sym. $\sigma_{ij} \epsilon_{kl}$

$C_{ijkl} = C_{jikl} = C_{klij} = C_{lki}$
 $C_{ijkl} = C_{klij}$ hyperelastic

$$E(u) : \delta(u) = \epsilon_{11}(u) \delta_{11}(u) + \epsilon_{22}(u) \delta_{22}(u) + 2\epsilon_{12}(u) \delta_{12}(u)$$

Voigt notation:

$$\gamma = \begin{bmatrix} \epsilon_{11} \\ \epsilon_{22} \\ 2\epsilon_{12} \\ \delta_{12} \end{bmatrix} \quad \delta = \begin{bmatrix} \sigma_{11} \\ \sigma_{22} \\ \sigma_{12} \end{bmatrix}$$



$$S(u) : \gamma(u) = \sigma_{11}(u) \epsilon_{11}(u) + \sigma_{22}(u) \epsilon_{22}(u) + 2\epsilon_{12}(u) \sigma_{12}(u)$$

they make

$$S_{3 \times 1} = C_{3 \times 3} \gamma_{3 \times 1}$$

↓
Voigt stiffness

if hyperelastic
 $C_{3 \times 3}$ is symmetric

The factor of 2 in Voigt shear "vector" is for a) s.gamma being 2 times strain energy density b) getting a symmetric C Voigt when material is hyperelastic

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$$\begin{bmatrix} u_1(x_1, x_2) \\ u_2(x_1, x_2) \end{bmatrix} = \begin{bmatrix} N_1^f & 0 & \dots & N_{n_f}^f(x_1, x_2) & N_1^p(x_1, x_2) & 0 & \dots & N_{n_p}^p(x_1, x_2) \\ 0 & N_1^f & \dots & N_{n_f}^f(x_1, x_2) & 0 & N_1^p(x_1, x_2) & \dots & N_{n_p}^p(x_1, x_2) \end{bmatrix} \begin{bmatrix} a_1 \\ a_2 \\ \vdots \\ a_{n_f} \\ a_{n_f+1} \\ a_{n_f+2} \\ \vdots \\ a_{n_f+n_p} \end{bmatrix} = \mathbf{N}$$

$$\begin{bmatrix} \delta_1(x_1, x_2) \\ \delta_2(x_1, x_2) \\ \delta_3(x_1, x_2) \end{bmatrix}, \quad \begin{bmatrix} \delta_{11} \\ \delta_{22} \\ \delta_{12} \end{bmatrix} = \begin{bmatrix} u_{,1} \\ v_{,2} \\ u_{,2} + v_{,1} \end{bmatrix} = \begin{bmatrix} \frac{\partial}{\partial x_1} & 0 \\ 0 & \frac{\partial}{\partial x_2} \\ \frac{\partial}{\partial x_2} & \frac{\partial}{\partial x_1} \end{bmatrix} \begin{bmatrix} u_1 \\ v_2 \end{bmatrix}$$

3x2 \rightarrow going from displacement to strain

$$\mathbf{B} = \mathbf{L} \mathbf{N}$$

3x2n \quad 3x2 \quad 2x2n

displacement-to-strain matrix

$$\mathbf{B} = \begin{bmatrix} \frac{\partial N_1^f}{\partial x_1} & 0 & \dots & \frac{\partial N_{n_f}^f}{\partial x_1} & \frac{\partial N_1^p}{\partial x_2} & \dots & \frac{\partial N_{n_p}^p}{\partial x_2} \\ 0 & \frac{\partial N_1^f}{\partial x_2} & \dots & \frac{\partial N_{n_f}^f}{\partial x_2} & \frac{\partial N_1^p}{\partial x_1} & \dots & \frac{\partial N_{n_p}^p}{\partial x_1} \end{bmatrix}$$

$$\mathbf{B} \quad 3 \times 2n = \left[\mathbf{B}_f \mid \mathbf{B}_p \right] \quad \omega = \left(\mathbf{N}_f^t \right)_{n_f \times 2}$$

free nodes

$$\mathbf{N}_f = \left[\mathbf{N}_f^f \mid \mathbf{N}_f^p \right] \quad 2 \times n_f$$

$$\mathbf{N}_p = \left[\mathbf{N}_p^f \mid \mathbf{N}_p^p \right] \quad 2 \times n_p$$

$$\epsilon(\omega) = \left(\mathbf{B}_f^t \right)_{n_f \times 3}$$

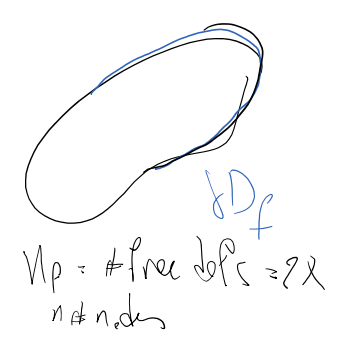
$$\epsilon(u) = \mathbf{B}_{3 \times n} a_{n \times 1}$$

② $u_i = N_a^i$
 $u = \mathbf{N}_{2 \times n} a_{n \times 1}$

$$\int_{\Omega} \rho \omega^t \dot{u} + \alpha \omega^t \dot{u} + \epsilon(\omega) \cdot \delta(u) \, dv = \int_{\Omega} \omega^t p b \, dv + \int_{\partial \Omega_f} \omega^t t \, ds$$

(5)

$$\int_{\Omega} \mathbf{N}_f^t \left[\mathbf{N}_f^f \mid \mathbf{N}_f^p \right] \begin{bmatrix} \dot{a}_f \\ \dot{a}_p \end{bmatrix} + \alpha \mathbf{N}_f^t \left[\mathbf{N}_f^f \mid \mathbf{N}_f^p \right] \begin{bmatrix} \dot{a}_f \\ \dot{a}_p \end{bmatrix} + \mathbf{B}_f^t \bar{\mathbf{C}}_{3 \times 3} \left[\mathbf{B}_f^f \mid \mathbf{B}_f^p \right] \begin{bmatrix} a_f \\ a_p \end{bmatrix} \, dv = \int_{\Omega} \mathbf{N}_f^t p b \, dv + \int_{\partial \Omega_f} \mathbf{N}_f^t t \, ds$$



$$+ B \quad C_{33} \quad \tilde{B}_f \quad B_p \quad \left(\begin{matrix} \text{Lap} \\ \text{op} \end{matrix} \right) \} dv = \int_D N_f^T \rho b dv + \int_{\partial D_f} N_f^T t ds$$

$$= \underbrace{\left(\int_D \rho N_f^T N_f dv \right)}_{M_{ff}} \ddot{a}_f + \underbrace{\left(\int_D \alpha N_f^T N_f dv \right)}_{C_{ff}} \dot{a}_f + \underbrace{\left(\int_D B_f^T C_{33} B_f dv \right)}_{K_{ff}} a_f + \underbrace{\int_D N_f^T \rho b dv}_{F_r} + \underbrace{\int_{\partial D_f} N_f^T t ds}_{F_N} - \underbrace{\left(\left(\int_D \rho N_f^T N_p dv \right) a_p + \left(\int_D \alpha N_f^T N_p dv \right) \dot{a}_p + \left(\int_D B_f^T C_{33} B_p dv \right) a_p \right)}_{F_D \text{ (Dirichlet force)}}$$

\downarrow source term force F_N (Neuman force)

$M \ddot{a} + C \dot{a} + K a =$	$F_r + F_N - F_D$ ODE where	(153a)
$a^f(t=0) = a_0^f, \dot{a}^f(t=0) = \dot{a}_0^f$	Initial condition (IC)	(153b)
$F_r = \int_D N^T \rho b dv$	Source term (body force) force vector	(153c)
$F_N = \int_{\partial D_f} N^T t ds$	Natural (Neumann) BC force vector	(153d)
$F_D = M^{fp} \ddot{a}_p + C^{fp} \dot{a}_p + K^{fp} a_p$	Essential (Dirichlet) BC force vector	(153e)
$M^{ff} = \int_D \rho N^T N^T dv,$	Mass matrices	(153f)
$M^{fp} = \int_D \rho N^T N^p dv$		

\rightarrow static part

$C^{ff} = \int_D \alpha N^T N^T dv,$	Damping matrices	(153g)
$C^{fp} = \int_D \alpha N^T N^p dv$		
$K^{ff} = \int_D B^T \bar{C} B dv,$	Stiffness matrices	(153h)
$K^{fp} = \int_D B^T \bar{C} B^p dv$		

Often, (153b) are written in the short form,

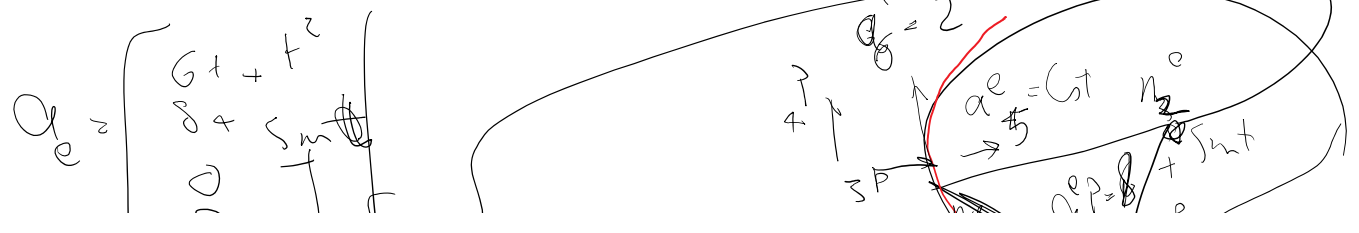
$$\underline{M} \ddot{a} + \underline{C} \dot{a} + \underline{K} a = F_r + F_N - F_D \quad \text{ODE where} \quad (154a)$$

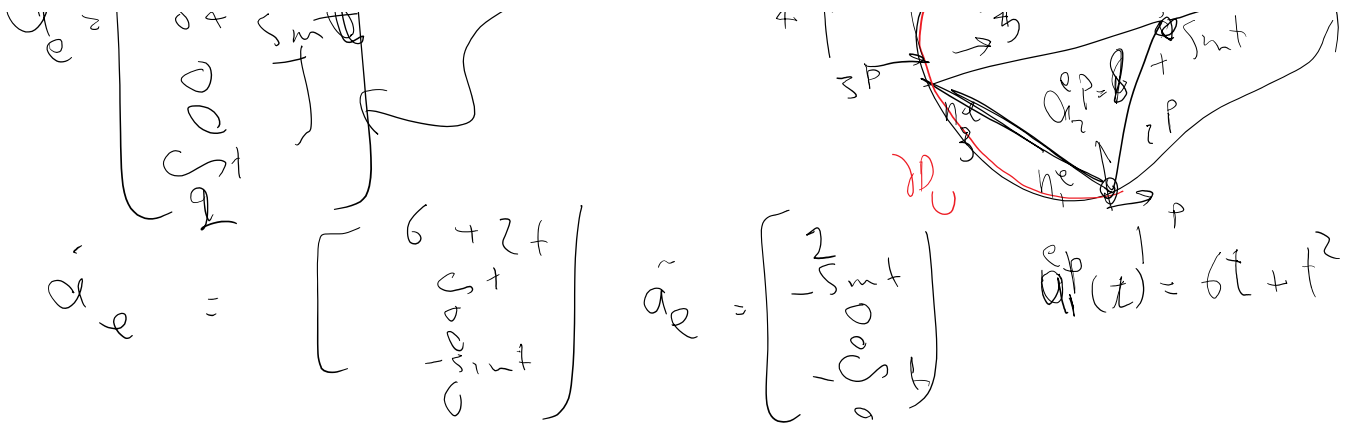
$$a(t=0) = a_0, \quad \dot{a}(t=0) = \dot{a}_0 \quad \text{Initial condition (IC)} \quad (154b)$$

where for short the superscripts for free dofs is dropped knowing that free and prescribed dofs are handles according to (153).

We already know how to do assembly. All terms (M, C, K, Fr, FN) can directly be formed with similar formulas at the element level and assembled to the global system.

For FD we do the calculation at the element level





$$\begin{aligned}
 & K_{6 \times 6}^e \quad C_{6 \times 6}^e \quad M_{6 \times 6}^e \\
 & F_D^p = K^p a^p + C^p \dot{a}^e + M^p \ddot{a}^e \\
 & F^p = F_r^p + F_w^p - F_D^p \\
 & \text{assemble to global system}
 \end{aligned}$$

things added to F_D in dynamic problem

As we will see through the following example, we actually DO NOT form $M^{j/p}$, $C^{j/p}$, $K^{j/p}$ directly, rather computing their corresponding values from elements and assemble their effects to global force F_D . Local versions of F_D is

$$f_D^e = M^e \ddot{a}^e + C^e \dot{a}^e + k^e a^e \quad (155)$$

where a^e is the local displacement vector of element formed by

1. Having zero values for free dofs.
2. Having prescribed values for prescribed dofs.

Damping matrix

$$M = \int_D \rho N_i^t N_j^e dv \quad C = \int_D \alpha N_i^t N_j^e dv$$

assume α, ρ are constant

$$M = \rho \int_D N^t N dv \quad C = \alpha \int_D N^t N dv$$

$$\alpha(x) = \rho(x) \bar{\alpha} \quad \text{constant}$$

$$C = \bar{\alpha} M$$

be cause of resistance of material to velocity $F = v \rightarrow F = -\alpha v$

$$\sigma = \alpha \dot{\epsilon}$$

we convert $\dot{\epsilon}$ to velocity

$$\int \sigma \dot{\epsilon} = -\alpha \dot{V}$$

$$\sigma = E \epsilon + E' \dot{\epsilon}$$

viscous material

Fourier transform

$$\sigma = E \dot{\epsilon} + E' j\omega \epsilon$$

$$\dot{\sigma} = \left(E + E' j\omega \right) \dot{\epsilon}$$

real part of stiffness \rightarrow imaginary part of stiffness

$\frac{E'}{E} \approx 0.01$ in frequency domain

$$K a \rightarrow K a + \left(\frac{E'}{E} K \right) \dot{a}$$

$$C = \beta K$$

from viscosity

$$C = \alpha M + \beta K$$

\downarrow
 damp $\alpha(x) = \bar{\alpha} \rho(x)$

\downarrow
 $E(x) = \beta \bar{E}(x)$
 \downarrow
 viscous stiffness

• Rayleigh damping matrix, generalizes the formula for C from (153g) by basically adding a coefficient of stiffness matrix. That is,

$$C = \alpha M + \beta K \tag{156}$$

- Justification for α is as before by modeling the equation of motion as in (128), that is $\rho b \rightarrow \rho b - \alpha v$ and getting (129) which is,

$$\dot{p} - \nabla \cdot \sigma + \alpha v - \rho b = 0$$

- Justification for β is modifying equation of motion in the form,

$$\sigma = C(\epsilon + \beta \dot{\epsilon}) \tag{157}$$