
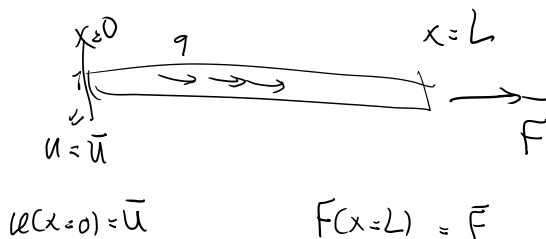


① $\frac{d}{dx} (EA \frac{du}{dx}) + q = 0$
 Final form of differential equation
 $(EAU)' + q = 0$
 $x \in (a, b)$ 

Balance law $\Sigma F = 0$

Can we solve ① now?

we need Boundary conditions (BCs)?



$F(x=L) = F$
 \downarrow
 $A\sigma = A(E\varepsilon) = F$
 \downarrow
 ε
 \downarrow
 $AE u'(L) = F$

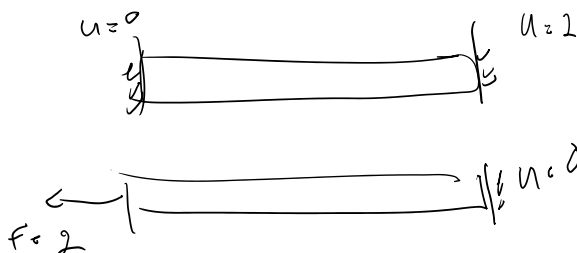
PDE + BCs

Boundary value Problem (BVP) } static (steady state)

other example

for dynamics + Initial conditions (IC)

Initial Boundary value Problem (IBVP)



Continue on BVP for a bar problem

Bar Problem ②

PDE $(EAU)' + q = 0$

M = order of differential eqn

$= 2$

$m = \frac{M}{2} = 1$ half the order of DE.

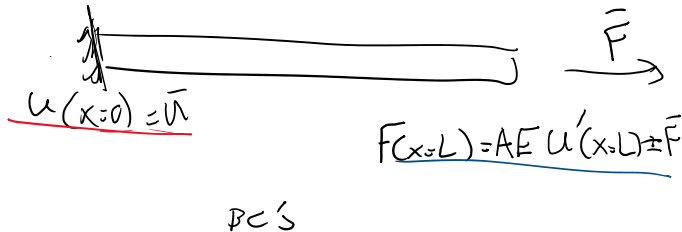
derivative order

$u(x=0) = \bar{u}$ (order 0 ... order $(\frac{m}{2})$)

Dirichlet BC

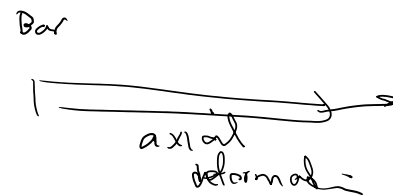
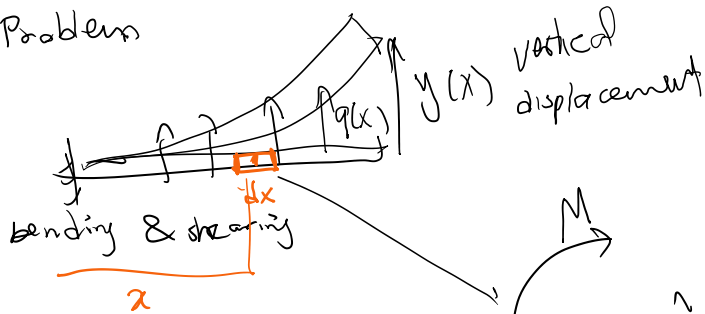
Solid mechanics

Kinematic



	derivative order	$u(x=0) = \bar{u}$ (order 0 ... order L)	Dirichlet BC Essential BC	Kinematic quantities (u)
$(M-1)$	L	$F(x=L) = EAu(x=L) = \bar{F}$	Neumann BC Natural BC	force-like quantities (F)
$M=2$	$(EAu)'' + q = 0$	$\int \int Df$	order $\frac{n}{2}$ to $M-1$	general force spatial flux

Beam Problem



Balance laws:

⊗ $\sum M_o = 0$

$-(V+dv) \frac{dx}{2} = V \frac{dx}{2}$

$+ (M+dM) - M = 0 \rightarrow$

divide by dx

balance of angular momentum

$dM = V dx + \frac{dV dx}{2}$

$\frac{dM}{dx} = V + \frac{dV}{2}$

Higher order mods (HOM)

$\frac{dM}{dx} = V$ (i)

⊗ $\sum F_y = 0 \Rightarrow$

Balance of forces

$dV = q dx$

$-(V+dv) + V + q(x) dx = 0$

\rightarrow

$\frac{dV}{dx} = q$ (ii)

$\frac{dV}{dx} = \frac{d}{dx} \left(\frac{dM}{dx} \right) = q \rightarrow$

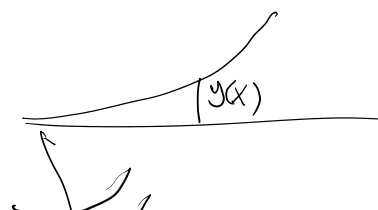
$\frac{d^2 M}{dx^2} = q$ ($M'' = q$) (3)

Differential eqn in terms of M

(3) \rightarrow DE in terms of y

For Euler Bernoulli Beam model:

constitutive eqn $M = EI \kappa$; $\kappa =$ curvature (4e)



constitutive eqn $M = EI \kappa$; $\kappa = \text{curvature}$ (4a)

kinematic eqn $\kappa = y''$

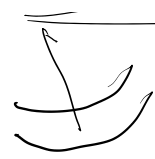
compatibility eqn

4a, 4b \rightarrow $M = EI y''$ (5)

(3) & (5) \Rightarrow

$$\frac{d}{dx^2} M = \frac{d}{dx^2} (EI \frac{d^2 y}{dx^2}) = q$$

or shorthand $(EI y'')'' = q$



compare force with bar problem

$F = \sigma A$
 $\sigma = E \epsilon$ } $F = EA \epsilon$ Constitutive eqn
 Compatibility eqn $\epsilon = u'$

(6) Differential eqn for beams

$M = 4 \rightarrow m = \frac{M}{2} = 2$

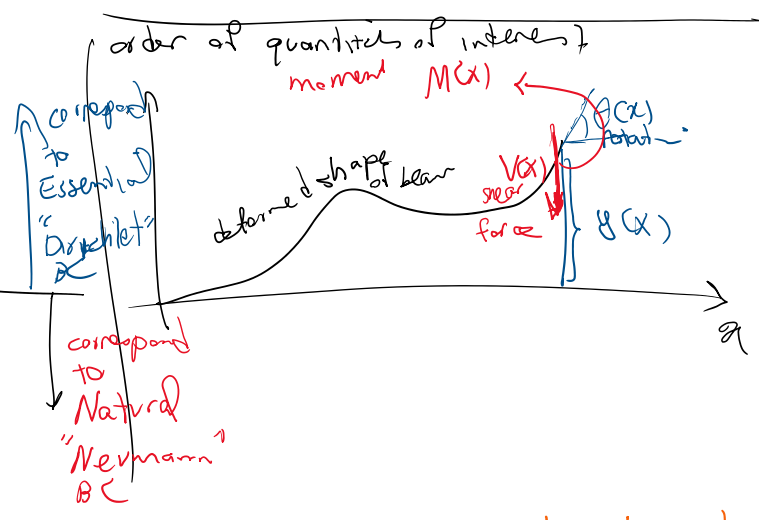
order 0) $y(x)$

1) $\theta(x) = y'(x)$

2) $M(x) = EI y''(x)$

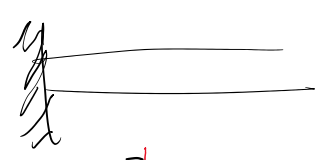
3) $V(x) = \frac{d}{dx} M = (EI y''')'$

DE is order $M=4$



We have m boundary conditions for each pt on the boundary

- essential
 - natural

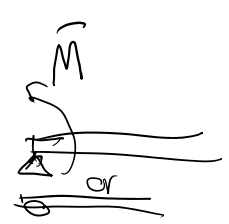


$y(x=0) = \bar{y}$
 $\theta(x=0) = \bar{\theta}$

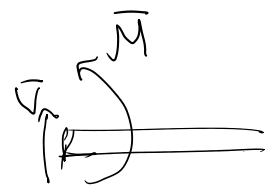
all essential



$(EI y'')' = V(x=0) = \bar{V}$
 $\theta(x=0) = \bar{\theta}$



$y(x=L) = \bar{y}$
 $EI y'' = M(x=L) = \bar{M}$

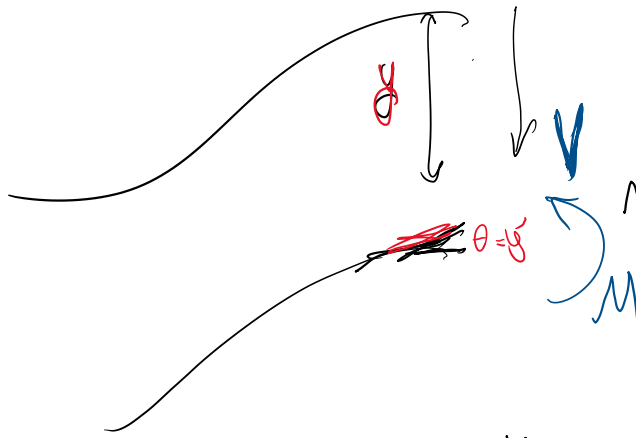


$(EI y') = V(x=L) = \bar{V}$
 $EI y'' = M(x=L) = \bar{M}$

all natural



work = $y(-V) = -yV$



$$\text{work} = y(-V) = -yV$$

$$M-1 = 3 = \begin{matrix} y & \longleftrightarrow & V = (EIy'')' \\ \text{order 0} & + & \text{order 3} \\ \text{work} & & \theta M \end{matrix}$$

$$M-1 = 3 = \begin{matrix} y' = \theta & \longleftrightarrow & M = EIy'' \\ \text{order 1} & + & \text{order 2} \end{matrix}$$

order

0	y	
1	$\theta = y'$	
2	$M = EIy''$	
$M-1 = 3$	$V = \frac{dM}{dx} = (EIy'')'$	

$(EIy'')'' + q = 0$

work/energy pairs



can we have this B.C.P?

take aways

we have $m = \frac{M}{2}$ BCs at each point
on the boundary of domain.

order 0 \longleftrightarrow order $M-1$
1 \longleftrightarrow $M-2$
:
:

only one from each can be specified

order 0 to $m-1$
Dirichlet / Essential

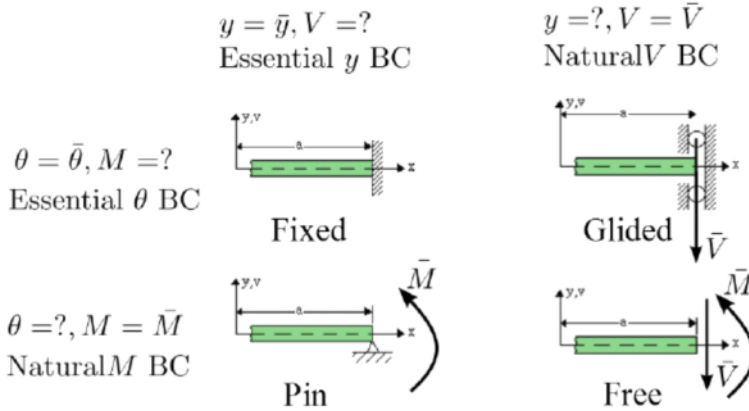
order m to $M-1$
Neumann / Natural

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Euler Bernoulli beam: BCs

Operator	Sample	elastostatics	operator order
$L_{2m}(u) = r$	$\frac{d^2 EI}{dx^2} \left(\frac{d^2 y}{dx^2} \right) = q$	$\frac{d^2 EI}{dx^2} \left(\frac{d^2 (\cdot)}{dx^2} \right) = q$	$m = 2(M = 4)$
$L_u(u) = u$	$u = \begin{bmatrix} \theta \\ y \end{bmatrix} = \begin{bmatrix} \frac{dy}{dx} \\ y \end{bmatrix} = \begin{bmatrix} \bar{\theta} \\ \bar{y} \end{bmatrix} = \bar{u}$	$L_u = \begin{bmatrix} \frac{d(\cdot)}{dx} \\ (\cdot) \end{bmatrix}$	$M_u = \begin{bmatrix} 1 \\ 0 \end{bmatrix}$
$L_f(u) = \bar{f}$	$\begin{bmatrix} EI \frac{d^2 y}{dx^2} \\ \frac{d}{dx} \left(EI \frac{d^2 y}{dx^2} \right) \end{bmatrix} = \begin{bmatrix} \bar{M} \\ \bar{V} \end{bmatrix}$	$L_f = \begin{bmatrix} EI \frac{d^2 (\cdot)}{dx^2} \\ \frac{d}{dx} \left(EI \frac{d^2 (\cdot)}{dx^2} \right) \end{bmatrix}$	$M_f = \begin{bmatrix} 2 \\ 3 \end{bmatrix}$

- One and only one of the pair M (Neumann) and θ (Dirichlet) is enforced at each end of the beam.
- One and only one of the pair V (Neumann) and y (Dirichlet) is enforced at each end of the beam.
- Neumann boundary conditions correspond to the flux terms (M and V).
- Neumann boundary conditions fall in the upper half of derivatives ($[m, 2m - 1] = [2, 3]$).
- Dirichlet boundary conditions fall in the lower half of derivatives ($[0, m - 1] = [0, 1]$).
- There are two boundary conditions at each end point (equal to $m = M/2$).
- $M_u + M_f = M - 1$.



- For each pair of Neumann and Dirichlet (Natural and Essential) boundary conditions, one and only one is specified.

2D elasticity

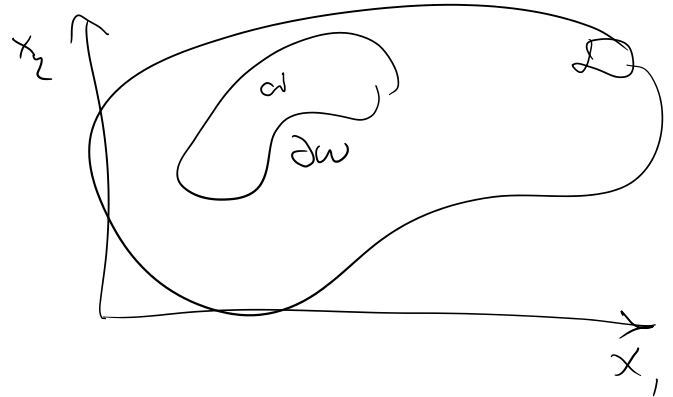
— Balance law

$$\forall w \quad \int_{\partial \omega} \sigma_{ij} n_j ds + \int_{\omega} \rho b_i dv = 0$$

Divergence theorem

$$\int_{\omega} \nabla_j \sigma_{ij} dv + \int_{\omega} \rho b_i dv = 0 \quad \forall w \quad \int_{\omega} (\nabla_j \sigma_{ij} + \rho b_i) w dv = 0$$

↓ (localize $\forall w \dots$)



⑧

PDE $\nabla_j \sigma_{ij} + \rho b_i = 0$

$$\sigma = \begin{bmatrix} \sigma_{11} & \sigma_{12} \\ \sigma_{21} & \sigma_{22} \end{bmatrix} \quad b = \begin{bmatrix} b_1 \\ b_2 \end{bmatrix}$$

$\sigma_{21} = \sigma_{12}$

eq1 $\left\{ \begin{aligned} \sigma_{11,1} + \sigma_{12,2} + \rho b_1 &= 0 \end{aligned} \right.$

eq2 $\left\{ \begin{aligned} \sigma_{21,1} + \sigma_{22,2} + \rho b_2 &= 0 \end{aligned} \right.$

eq = 2

unknowns = 3
stresses

$$s = \begin{pmatrix} \sigma_{11} \\ \sigma_{22} \\ \sigma_{12} \end{pmatrix}$$

Voigt notation

1D bar

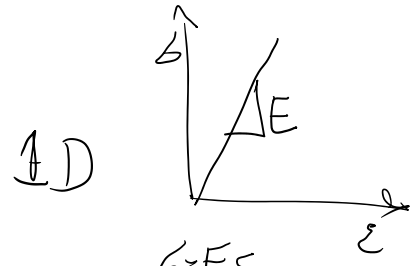
$F_{,x} + q = 0$

$(A\sigma)_{,x} + q = 0$

$\# \text{ eqns} = 2$ $\# \text{ unknowns} = 3$ stresses	$S = \begin{pmatrix} \sigma_{11} \\ \sigma_{22} \\ \sigma_{12} \end{pmatrix}$	Vagt notation
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We need to close this system

— Constitutive equation



2D

$$\begin{bmatrix} \sigma_{11} \\ \sigma_{22} \\ \sigma_{12} \end{bmatrix} = \underbrace{C}_{3 \times 3} \begin{bmatrix} \epsilon_{11} \\ \epsilon_{22} \\ 2\epsilon_{12} \end{bmatrix}$$

elasticity stiffness matrix

$$\sigma = E \epsilon$$

3 eqns added

3 unknowns added ($\epsilon_{11}, \epsilon_{22}, \epsilon_{12}$)

— Compatibility equation

$$\epsilon = \begin{bmatrix} \epsilon_{11} & \epsilon_{12} \\ \epsilon_{21} & \epsilon_{22} \end{bmatrix} = \frac{\nabla u + (\nabla u)^T}{2} = \frac{1}{2} \begin{bmatrix} u_{1,1} & u_{1,2} \\ u_{2,1} & u_{2,2} \end{bmatrix} +$$

$$\begin{bmatrix} u_{11} & v_{21} \\ u_{12} & v_{22} \end{bmatrix} \Rightarrow \begin{cases} \epsilon_{11} = u_{1,1} \\ \epsilon_{22} = u_{2,2} \\ \epsilon_{12} = \epsilon_{21} = \frac{u_{1,2} + u_{2,1}}{2} \end{cases}$$

2 unknowns added u_1, u_2
 3 eqns added