

From last time:

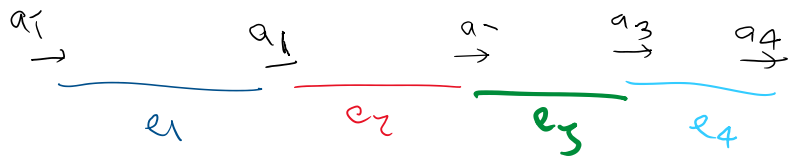
Calculate e_1

$$f_D^{e_1} = k a^{e_1} = \begin{bmatrix} 2 & -2 \\ -2 & 2 \end{bmatrix} \begin{bmatrix} 1 \\ 0 \end{bmatrix}$$

$$= \begin{bmatrix} 2 \\ -2 \end{bmatrix}$$

$$f_r^{e_1} = r^{e_1} q^{e_1}$$

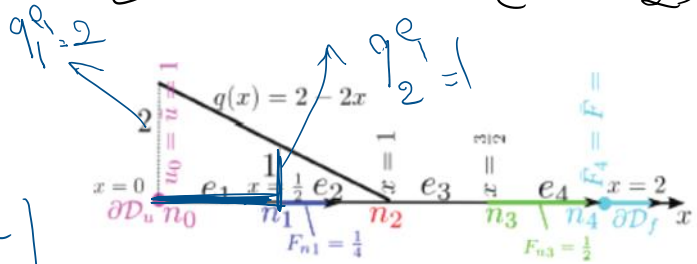
$$f_r^{e_1} = \frac{1}{12} \begin{bmatrix} 2 & 1 \\ 1 & 2 \end{bmatrix} \begin{bmatrix} 2 \\ 1 \end{bmatrix} = \begin{bmatrix} 5/12 \\ 4/12 \end{bmatrix}$$



$$M(e_1) = [1 \ 1]$$

$$a^{e_1} = \begin{bmatrix} a_1 \\ a_2 \end{bmatrix} = \begin{bmatrix} 1 \\ 0 \end{bmatrix}$$

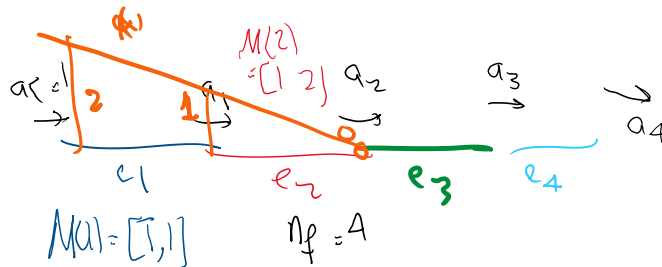
$$r^{e_1} = \frac{L e_1}{6} \begin{bmatrix} 2 & 1 \\ 1 & 2 \end{bmatrix} = \frac{1}{12} \begin{bmatrix} 2 & 1 \\ 1 & 2 \end{bmatrix}$$



last time

$$f_D^{e_1} = f_r^{e_1} + f_D^{e_1} - f_D^{e_1} = \begin{bmatrix} 5/12 \\ 4/12 \end{bmatrix} - \begin{bmatrix} 2 \\ -2 \end{bmatrix} = \begin{bmatrix} -19/12 \\ 28/12 \end{bmatrix}$$

0 1D elements



$$F = \begin{bmatrix} 1 \\ 2 \\ 3 \\ 4 \end{bmatrix} = \begin{bmatrix} 28/12 + 2/12 \\ 1/12 \\ \\ \end{bmatrix}$$

Element 2 $f_D^{e_2} = \begin{bmatrix} 0 \\ 0 \end{bmatrix}$

$$f_r^{e_2} = f_r^{e_2} = r^{e_2} \begin{bmatrix} a_1 \\ a_2 \end{bmatrix} = \frac{1}{6} \begin{bmatrix} 2 & 1 \\ 1 & 2 \end{bmatrix} \begin{bmatrix} 1 \\ 0 \end{bmatrix} = \frac{1}{12} \begin{bmatrix} 2 & 1 \\ 1 & 2 \end{bmatrix} \begin{bmatrix} 1 \\ 0 \end{bmatrix} = \begin{bmatrix} 2/12 \\ 1/12 \end{bmatrix}$$

$$f_r^e = f_r^{e2} = r^{e2} \begin{bmatrix} q_1 \\ q_2 \end{bmatrix} = \frac{1}{6} \begin{bmatrix} 2 & 1 \\ 1 & 2 \end{bmatrix} \begin{bmatrix} q_1 \\ q_2 \end{bmatrix} = \frac{1}{12} \begin{bmatrix} 2 & 1 \\ 1 & 2 \end{bmatrix} \begin{bmatrix} 1 \\ 0 \end{bmatrix} = \begin{bmatrix} 1/12 \\ 1/12 \end{bmatrix}$$

$$f^e = f_r^e + f_n^e - f_D^e$$

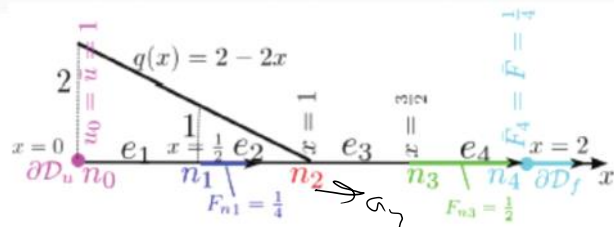
$$f^e = \begin{bmatrix} 0 \\ 0 \end{bmatrix} \quad f^e = \begin{bmatrix} 0 \\ 0 \end{bmatrix}$$

$$F_e = \begin{bmatrix} 39/12 \\ 1/12 \\ 0 \\ 0 \end{bmatrix}$$

$$F_n = \begin{bmatrix} 1/4 \\ 0 \\ 1/2 \\ 1/4 \end{bmatrix}$$

$$F = F_e + F_n = \begin{bmatrix} 11/4 \\ 1/12 \\ 1/2 \\ 1/4 \end{bmatrix}$$

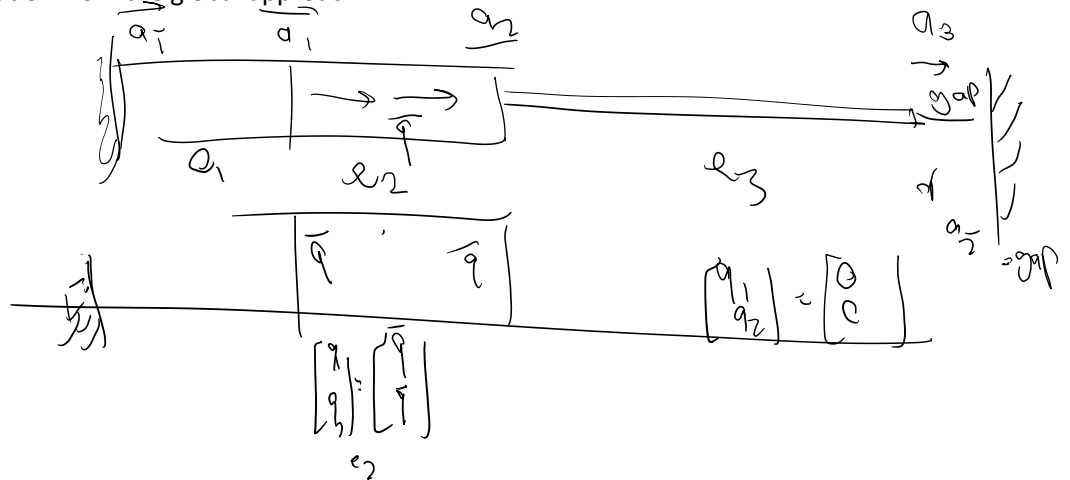
$$a = K^{-1} F = \begin{bmatrix} 43/24 \\ 53/24 \\ 31/24 \\ 65/24 \end{bmatrix} \rightarrow a_2$$



This matches the solution from the global approach.

HW 4

9/11

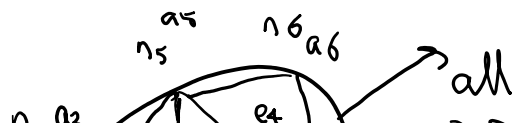


Why element forces assemble the way we saw above

At the element level we have 3 forces:

$$f^e = f_r^e + f_n^e - f_D^e$$

Source Neuman Dirichlet



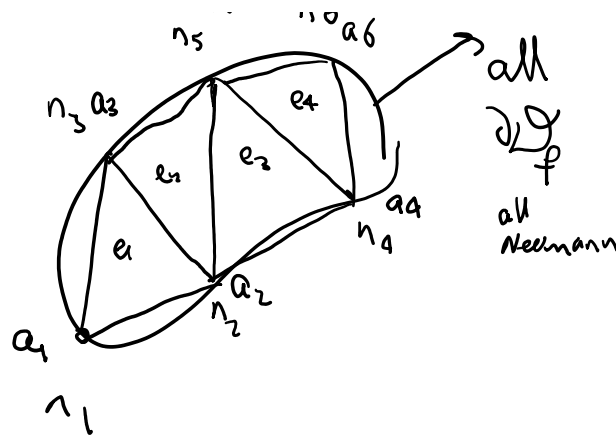
Source \downarrow Neumann \downarrow Dirichlet

$p = 6$

$p = 0$ (---)

$$F_p = \int \begin{bmatrix} N_1 \\ N_2 \\ N_3 \\ N_4 \\ N_5 \\ N_6 \end{bmatrix} Q dv$$

heat conduction problem:
 $r = Q$
 $\int N^T Q dv$



20 heat conduction
 T (temperature) unknown
 1 dof/node

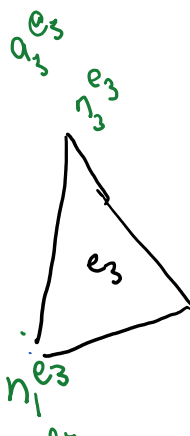
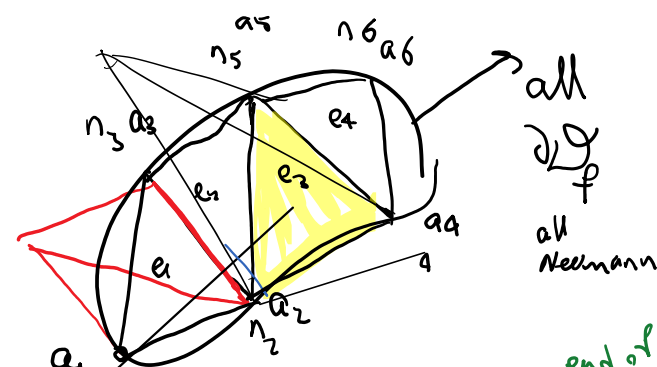
$$F_r^{e3} = \int_{e3} \begin{bmatrix} N_1 \\ N_2 \\ N_3 \\ N_4 \\ N_5 \\ N_6 \end{bmatrix} Q dv$$

$$= \int_{e3} \begin{bmatrix} 0 \\ N_1 \\ N_2 \\ N_3 \\ N_4 \\ N_5 \\ 0 \end{bmatrix} Q dv$$

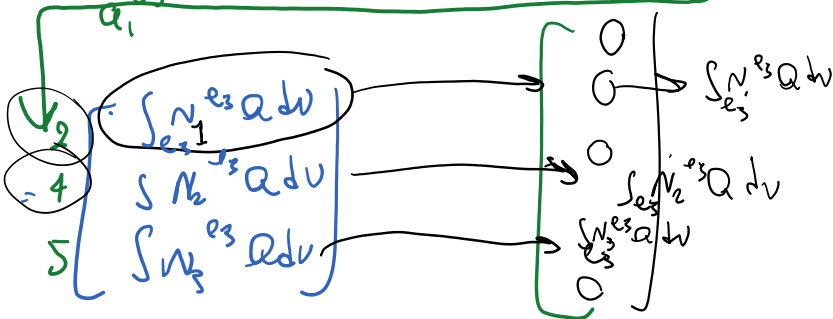
$$= \int_{e3} \begin{bmatrix} 0 \\ N_1^{e3} \\ 0 \\ N_2^{e3} \\ N_3^{e3} \\ 0 \end{bmatrix} Q dv$$

$$p^{e3} = \int \begin{bmatrix} N_1^{e3} \\ N_2^{e3} \\ N_3^{e3} \end{bmatrix} Q dv$$

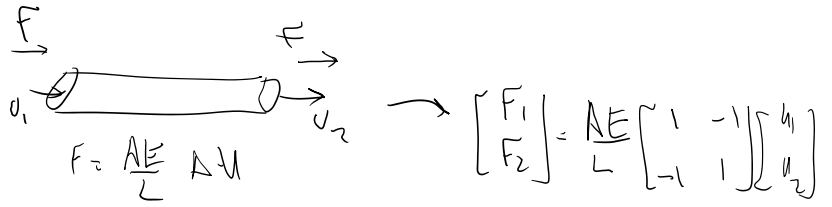
the local p^e assembled to global F_r matches contributions of element e



LE $M_{e3} = [2, 4, 5]$
 $M_{e3} = [2, 4, 5]$
 nodal map
 end of course project text input



Calculating the stiffness matrix for a 1D bar
 In the very first class I demonstrated



General formula for K in these cases ACM or $E(x)$



$$\begin{cases} N_1^e(\xi = -1) = 1 \\ N_1^e(\xi = 1) = 0 \end{cases}$$

$$N_1^e(\xi) = a_0 + a_1 \xi + a_2 \xi^2 \dots$$

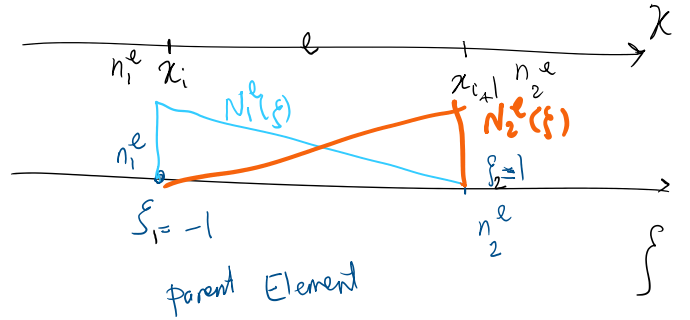
sufficient

$$N_1^e(\xi = -1) = a_0 - a_1 = 1$$

$$N_1^e(\xi = 1) = a_0 + a_1 = 0$$

$$a_0 = \frac{1}{2} \quad a_1 = -\frac{1}{2}$$

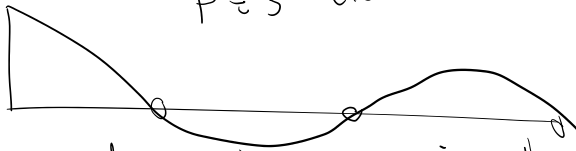
$$N_1^e(\xi) = \frac{1-\xi}{2}$$



Same process $N_2^e(\xi) = \frac{1+\xi}{2}$

Easier way (Lagrange polynomials)

Later $P=3$ element



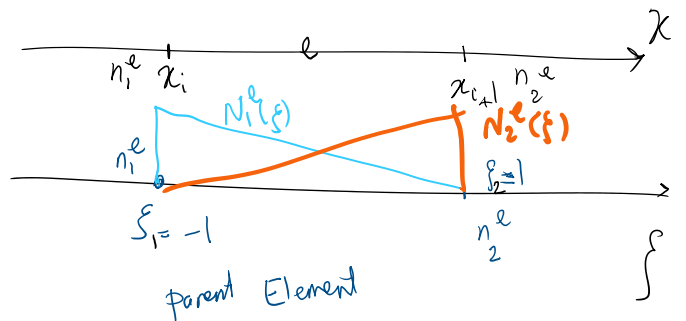
this approach right away gives the shape functions

$$N_1^e(\xi) = \frac{\xi - \xi_2}{\xi_1 - \xi_2} = \frac{\xi - (1)}{-1 - 1} = \frac{1-\xi}{2}$$

$$N_2^e(\xi) = \frac{\xi - \xi_1}{\xi_2 - \xi_1} = \frac{\xi - (-1)}{1 - (-1)} = \frac{\xi+1}{2}$$

$$K^e = \int_{x_i}^{x_{i+1}} B^e T D B^e dx$$

①



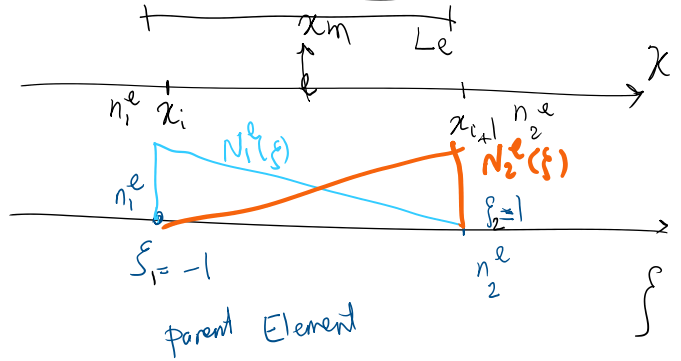
$$N^e = [N_1^e \quad N_2^e] \quad B^e = \frac{d}{dx} [N_1^e(\xi) \quad N_2^e(\xi)] = \frac{d}{d\xi} [N_1^e(\xi) \quad N_2^e(\xi)] \cdot \frac{d\xi}{dx}$$

B^e

$$B = \frac{1}{J} B^e \quad (2)$$

we need $x(\xi)$

$$K^e = \int_{\xi=-1}^{\xi=1} B^T D B^e \left(\frac{dx}{d\xi} d\xi \right) \quad (3)$$



$$\xi_1 = -1 \rightarrow x_i$$

$$\xi_2 = 1 \rightarrow x_{i+1}$$

$$x(\xi) = a_0 + a_1 \xi$$

$$a_0 + a_1(-1) = x_i$$

$$a_0 + a_1(1) = x_{i+1}$$

$$a_0 = \frac{x_i + x_{i+1}}{2} = x_m$$

$$a_1 = \frac{x_{i+1} - x_i}{2} = \frac{L_e}{2}$$

$$x(\xi) = x_m + \frac{L_e}{2} \xi \quad (4)$$

$$J = \frac{dx}{d\xi} = \frac{L_e}{2}$$

$$(3) : (2) \Rightarrow K^e = \int_{\xi=-1}^{\xi=1} \left(\frac{1}{J} \begin{bmatrix} B_{\xi_1} \\ B_{\xi_2} \end{bmatrix} \right) \underbrace{EA(\xi)}_{D(\xi)} \left(\frac{1}{J} \begin{bmatrix} B_{\xi_1} & B_{\xi_2} \end{bmatrix} \right) J d\xi$$

$$K^e = \int_{\xi=-1}^{\xi=1} \frac{EA(\xi)}{J} \begin{bmatrix} B_{\xi_1} \\ B_{\xi_2} \end{bmatrix} \begin{bmatrix} B_{\xi_1} & B_{\xi_2} \end{bmatrix} d\xi = \int_{\xi=-1}^{\xi=1} \frac{EA(\xi)}{J} \begin{bmatrix} -\frac{1}{2} \\ \frac{1}{2} \end{bmatrix} \begin{bmatrix} -\frac{1}{2} & \frac{1}{2} \end{bmatrix} d\xi \Rightarrow$$

note

$$N = [N_1 \ N_2] = \begin{bmatrix} \frac{1-\xi}{2} & \frac{1+\xi}{2} \\ -\frac{1}{2} & \frac{1}{2} \end{bmatrix}$$

$$B_{\xi} = \frac{dN}{d\xi} = \begin{bmatrix} -\frac{1}{2} & \frac{1}{2} \end{bmatrix}$$

Finite Element
Stiffness
matrix

$$K^e = \frac{1}{2L^e} \int_{-1}^1 AE(\xi) d\xi \begin{bmatrix} +1 & -1 \\ -1 & 1 \end{bmatrix}$$

for constant AE

$$i.e. \quad K^e = \frac{AE}{L} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}$$

$$k^e = \left(\frac{AE}{L} \right)_e \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}$$

Side note: is any easier way to form $X(\xi)$?

1) $u(\xi) = a_1^e N_1(\xi) + a_2^e N_2(\xi)$ (sh)

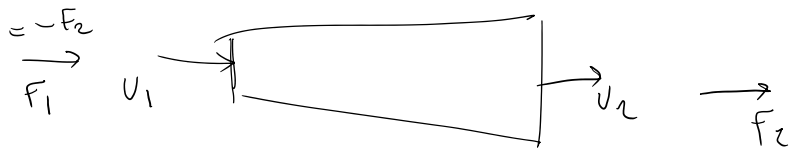
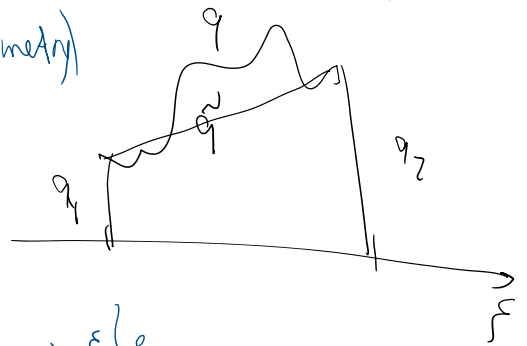
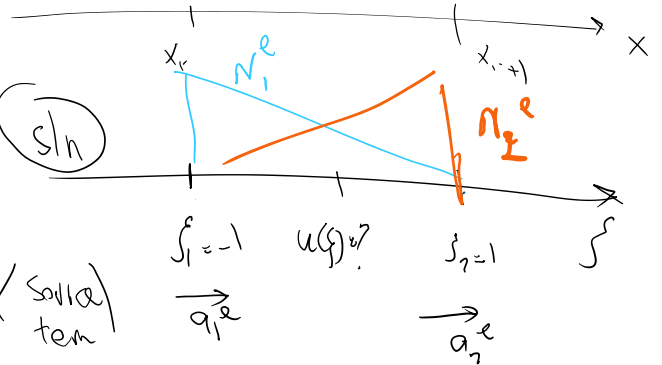
2) $q(\xi) = q_1 N_1(\xi) + q_2 N_2(\xi)$ (solid) / (tem)

3) $x(\xi) = x_1 N_1(\xi) + x_2 N_2(\xi)$ (geometry)

coordinate @ node

$$= x_i \left(\frac{1-\xi}{2} \right) + x_{i+1} \left(\frac{1+\xi}{2} \right)$$

$$= \frac{x_i + x_{i+1}}{2} + \xi \left(\frac{x_{i+1} - x_i}{2} \right) = x_m + \xi \frac{L_e}{2}$$



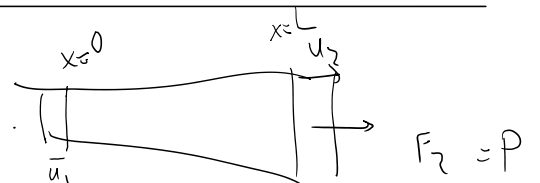
$$\begin{bmatrix} F_1 \\ F_2 \end{bmatrix} = \underbrace{\left(\int_{-1}^1 \frac{AE(\xi)}{2L_e} d\xi \right)}_{k^{FEM}} \begin{bmatrix} +1 & -1 \\ -1 & 1 \end{bmatrix} \begin{bmatrix} u_1 \\ u_2 \end{bmatrix} \Rightarrow F_2 = k^{FEM} (u_2 - u_1) = \Delta u$$

$$F_1 = -F_2$$

k^{FEM} stiffness is not exact for $AE \neq \text{constant}$

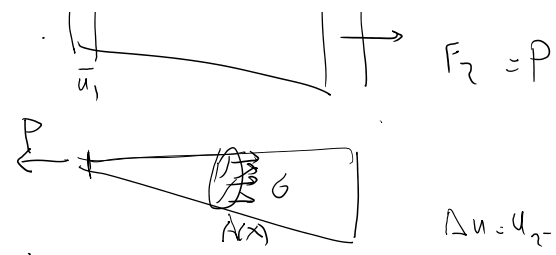
Exact solution

$$\dots = \int^L \epsilon dx \quad \left(\begin{matrix} -P = F_1 \\ \sigma = du \end{matrix} \right)$$



$$\Delta u_i = u_2 - u_1 = \int_0^L \epsilon dx \quad \left(\epsilon = \frac{du}{dx} \right)$$

$\int du = \int \epsilon dx$

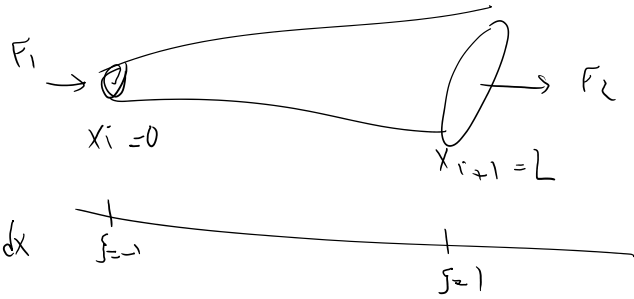


$$\Delta u = u_2 - u_1 = \int_0^L \epsilon dx = \int_0^L \left(\frac{\sigma}{E} \right) dx = \int_0^L \frac{P}{A(x)E(x)} dx$$

$\Delta u = u_2 - u_1$
 $\sigma = \frac{P}{A}$

$$\Rightarrow \Delta u = \left(\int_0^L \frac{dx}{AE(x)} \right) P \Rightarrow K^{exact} = \frac{P}{\Delta u} = \frac{1}{\int_0^L \frac{dx}{AE(x)}}$$

Comparisons



$$k^{FEM} = \frac{\int_{j=0}^1 A E(x) dx}{2L} = \frac{\int_0^L A E(x) dx}{L^2}$$

$$k^{exact} = \frac{1}{\int_0^L \frac{dx}{A E(x)}}$$

$$K^e = k \begin{bmatrix} +1 & -1 \\ -1 & 1 \end{bmatrix}$$

6

FEM σ_1

$$u = u_1 N_1 + u_2 N_2$$

$$\epsilon = \frac{du}{dx}$$

$$\sigma = E \epsilon$$

$$F = \sigma A$$

$$u = \int d\epsilon$$

$$\epsilon = \frac{\delta}{L}$$

$$\delta = \frac{P}{A}$$

$$P = \text{const}$$

Exact