

1. For steel the elastic modulus  $E = 210$  GPa. For a given specimen material strength  $\sigma_f = 250$ MPa. **(40 Points)**
  - (a) Provide an estimate for theoretical strength  $\sigma_c$  based on atomistic potential.
  - (b) What is the ratio  $\sigma_f/\sigma_c$  and what is the reason for discrepancy?
  - (c) Obtain an estimate for the ratio  $r = a/x_0$  for  $a =$  representative defect size in the material and  $x_0 =$  atomistic lattice length scale.
  - (d) If  $x_0 \approx 10^{-10}$  m obtain an estimate for surface energy  $\gamma_c$ .

2. For a specimen which can be characterized by a point force  $P$  and displacement  $u$  we have two measurements of  $(u_1, P_1)$  for crack length  $a_1$  and  $(u_2, P_2)$  for crack length  $a_2$ . The stiffness is defined as  $K = P/u$ . Recalling that  $G = -\frac{\partial \Pi}{\partial A} = -\frac{\partial \Pi}{\partial aB}$  for  $A$  crack surface,  $a$  crack length, and  $B$  crack (a) obtain an equation for  $G$  in terms of  $u$  and  $\frac{\partial K}{\partial a}$ , (b) Given that compliance  $C = 1/K$  show that  $G = \frac{P^2}{2B} \frac{\partial C}{\partial a}$ . **(70 Points)**

Hint: Note that  $K = \tan(\theta)$ ,  $G \approx -\frac{\Pi_2 - \Pi_1}{\Delta aB} = -\frac{U_{e2} - U_{e1} - W_{12}}{\Delta aB} = \frac{\text{Shaded area}}{\Delta aB}$ . Finally, express shaded area using  $\Delta\theta$ .

**Remark:** You do not need to show that  $G = \lim_{\Delta a \rightarrow 0} \text{shaded area} / (B\Delta a)$ . You need to show that the limit expression is equal to  $G = \frac{P^2}{2B} \frac{\partial C}{\partial a}$  by using the small angle approximation for  $\Delta\theta$ .

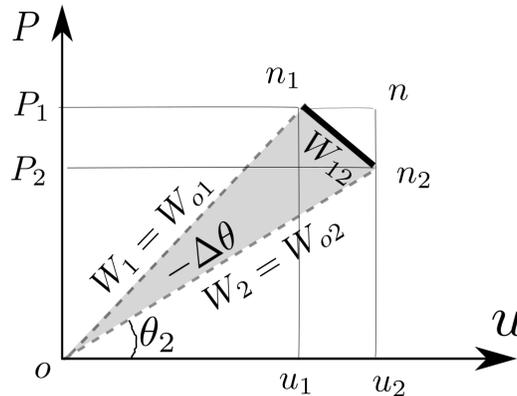


Figure 1: Force displacement relation for a point force system.

3. A composite laminate is made by bonding two long thin strips of steel with an adhesive epoxy, as shown in figure 2. A patch of the adhesive was intentionally left out in the central section in order to create a central crack of length  $2a$  in the bilayer plate. The joined plates are pulled apart by equal and opposite tensile forces,  $P$ . From beam theory, the deflection of a double cantilever beam of length  $L$  (half the crack length) under load  $P$  is  $\Delta = \frac{PL^3}{192EI} \Rightarrow$

$$\frac{\delta}{2} = \frac{P(2a)^3}{192EI} \Rightarrow \delta = \frac{Pa^3}{12EI}$$

where the crack opening  $\delta$  is the displacement of load  $P$ . Since there are two crack front we have,

$$2G = \frac{P^2}{2B} \frac{\partial C}{\partial a}$$

for  $C = \delta/P$ . The parameters for the problem are: initial crack length  $2a_0 = 60$  mm,  $E = 200$  GPa,  $H = 0.97$  mm and  $B = 10.1$  mm.

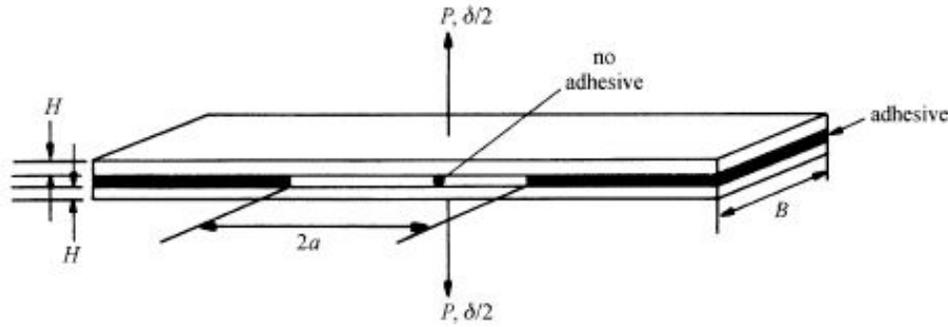


Figure 2: Geometry and loading of joined plates (image source: Suresh, Fatigue of materials)

Consider two different  $R$  (crack propagation resistance) equations:

$$R_1(a) = R_0 \quad (1a)$$

$$R_2(a) = \begin{cases} R_0 & a = a_0 \\ R_0 + \Delta R \left[ 1 - \left( \frac{a - a_0 - l}{l} \right)^2 \right] & a_0 < a < l + a_0 \\ R_0 + \Delta R & l + a_0 < a \end{cases} \quad (1b)$$

Basically  $R_1$  corresponds to a constant resistance and  $R_2$  is for a case that  $R$  increases from  $R_0$  from  $a = a_0$  to  $R_0 + \Delta R$  at  $a = a_0 + l$  by a parabolic equation that smoothly (zero slope at  $a = a_0 + l$ ) transitions to constant  $R = R_0 + \Delta R$  for larger crack lengths. The parameters  $\Delta R$  and  $l$  are strengthening of crack resistance due to crack propagation and a characteristic length respectively. Let  $R_0 = 300 \text{ Pa}\cdot\text{m}$ ,  $\Delta R = 1200 \text{ Pa}\cdot\text{m}$  and  $l = a_0 = 30 \text{ mm}$ . Answer the following **(140 Points)**:

- Plot R-Curve for  $R_1$  and  $R_2$  (Resistance  $R$  versus crack length  $a$ ).
- Consider load control and displacement control methods to initiate and cause crack propagation. In load control we increase  $P$  until at  $P_{ini}$  crack starts propagating. If a critical value  $P_{cr}$  exists once  $P = P_{cr}$  crack propagation becomes unstable, *i.e.*, propagates without increasing  $P$ . Same concepts apply for displacement control to  $\delta_{ini}$  and  $\delta_{cr}$  (if it exists). Add  $G$  curves for load control  $P = 45, 105, 130 \text{ N}$  and displacement control for  $\delta = 1 \text{ mm}$  and  $3 \text{ mm}$  to the same  $R$  plot that included  $R_1$  and  $R_2$  curves.
- For constant resistance given by  $R_1$  obtain  $P_{ini}^1, P_{cr}^1$  (if for any  $a$  crack becomes unstable) and for load control  $\delta_{ini}^1$  and  $\delta_{cr}^1$  (if for any  $a$  displacement control method results in unstable crack propagation).
- For increasing resistance case  $R_2$  obtain  $P_{ini}^2, P_{cr}^2$  and for load control  $\delta_{ini}^2$  and  $\delta_{cr}^2$ . Again, note that the critical values may not exist.
- Schematically add  $G$  curves for  $P_{ini}^2, \delta_{ini}^2, P_{cr}^2$  (if exists), and  $\delta_{cr}^2$  (if exists) to  $G, R$  plot from previous steps. Which of loadings eventually result in unstable crack propagation?
- For stable crack growth the relation between  $a, P$ , and  $\delta$  is obtained by  $G = R$  (crack force equal to crack resistance). For load control and displacement control solve for both  $P$  and  $\delta$  as functions of  $a$  for two different cases of resistance  $R_1$  and  $R_2$  ( $2 \times 2$  solutions) and generate the following plots: 1)  $P$  (vertical axes) plotted versus  $\delta$  (horizontal), 2)  $a$  versus  $P$ , and 3)(optional)  $a$  versus  $\delta$ . The explicit form of these relations are not needed and only the plots need to be correct.
- Based on all the generated plots compare load control and displacement control and also constant  $R$  versus increasing  $R$  cases.