

2D/3D elastostatics

Recall from last time



$$\forall \Omega \int_{\partial \Omega} \sigma \cdot n \, ds + \int_{\Omega} \rho b \, dV = 0$$

Apply divergence theorem

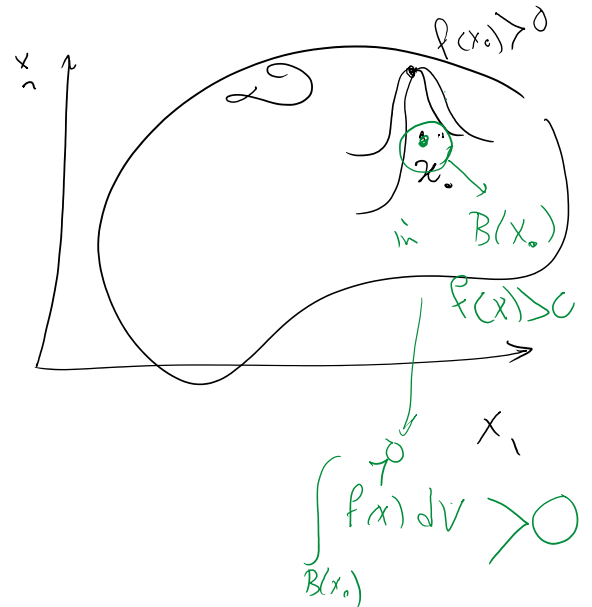
$$\int_{\Omega} \nabla \cdot \sigma \, dV + \int_{\Omega} \rho b \, dV = 0 \Rightarrow$$

$$\forall \Omega \int_{\Omega} (\underbrace{\nabla \cdot \sigma + \rho b}_f) \, dV = 0$$

Localization theorem:

- Let f be continuous in D
- $\forall \Omega \in D \int_{\Omega} f(x) \, dx = 0$
 like dV

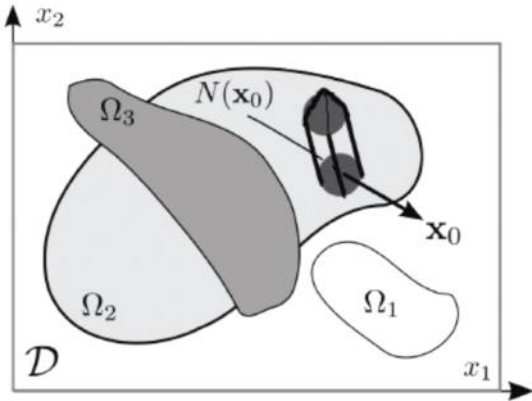
$$\forall x \in D \quad \underline{f(x) = 0}$$



Localization theorem

Localization theorem states that if the integral of a **continuous** function is zero for all subsets of \mathcal{D} , then the function is zero:

$$\forall \Omega \subset \mathcal{D} : \int_{\Omega} g(\mathbf{x}) \, dV = 0 \Rightarrow \forall \mathbf{x} \in \mathcal{D} : g(\mathbf{x}) = 0 \quad (21)$$



Let's assume $g(x_0) \neq 0$ (e.g., $g(x_0) > 0$). Since $g(\mathbf{x})$ is continuous, there is a neighborhood of \mathbf{x}_0 ($N(\mathbf{x}_0)$) that $g(\mathbf{x}) > 0$. We choose an Ω that is only nonzero inside $N(\mathbf{x}_0)$. Then, $\int_{\Omega} g(\mathbf{x}) \, dV > 0$. Thus, $g(\mathbf{x}_0)$ cannot be nonzero and the function g is identically zero.

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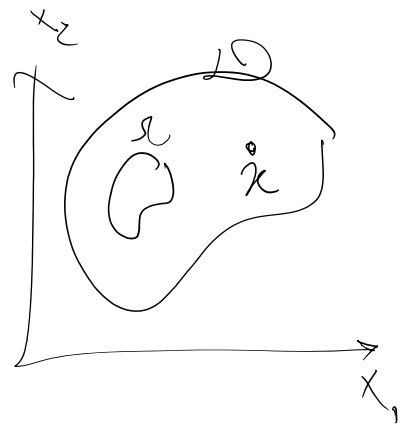
we can apply this to our balance law

$$\forall \Omega \subset \mathcal{D} \quad \int_{\Omega} (\nabla \cdot \mathbf{b} + \rho b) \, dV = 0$$

over "f" (or "g") volume

\implies

$$\textcircled{I} \quad \forall \mathbf{x} \in \mathcal{D} \quad \nabla \cdot \mathbf{b} + \rho b = 0$$



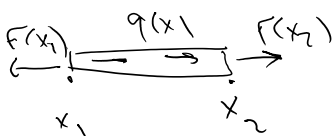
Strong form (the)
Partial Differential Equations
(PDE)

Strong form

$$\frac{dF}{dx} + \rho(x) = 0$$

$$\nabla \cdot \mathbf{b} + \rho b = 0$$


1D
2013D



Balance law

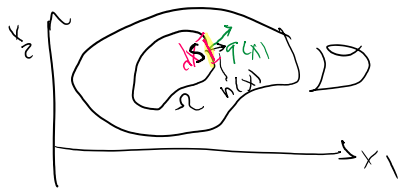
$$\forall x_1, x_2 \quad F(x_2) - F(x_1) + \int_{x_1}^{x_2} q(x) \, dx = 0$$

$$\forall \Omega \quad \int_{\partial \Omega} \mathbf{b} \cdot \mathbf{n} \, ds + \int_{\Omega} \rho b \, dV = 0$$

	$\int_V \rho \frac{D}{Dt} + \int_S \rho b \cdot n \, dS = 0$	$\nabla \cdot b + \rho b = 0$ $-\nabla \cdot f_x + r = 0$
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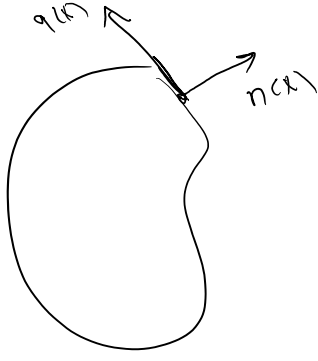
Heat conduction, T = temperature q = heat flux density Q = heat source	$\int_V \rho \frac{D}{Dt} + \int_S Q \cdot n \, dS = 0$	$-\nabla \cdot q + Q = 0$ $-\nabla \cdot f_x + r = 0$
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Balance of energy



energy flux through \int_S surface differential

$q \cdot n \, dS \rightarrow$ energy that goes out at



Most balance laws are in this form
 outward Spatial flux density f_x : > 0 it means the quantity (energy, mass, ...) decreases
 Source term: r



$$\int_V \rho \frac{D}{Dt} + \int_S f_x \cdot n \, dS + \int_V r \, dV = 0$$

balance law \Downarrow
 Divergence theorem

Divergence theorem

$$\int_{\Omega} (-\nabla \cdot f_x + r) dV = 0$$

② $\nabla \times$

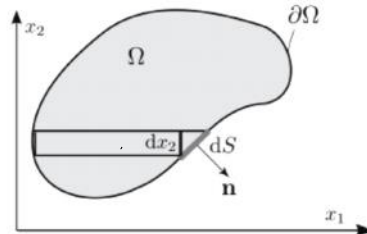
$$-\nabla \cdot f_x + r = 0$$

PDE (Strong form)

Fun fact for you

Transfer of boundary to interior integral higher dimensions

- Ω is compact and closed.
- $\partial\Omega$ is piecewise smooth.
- Normal vector \mathbf{n} is defined almost everywhere (a.e.) and is pointing outward.
- tensor field (scalar, vector, matrix, ...):
 - $F_{,i} = \partial F / \partial x_i$ exists everywhere and
 - is continuous.



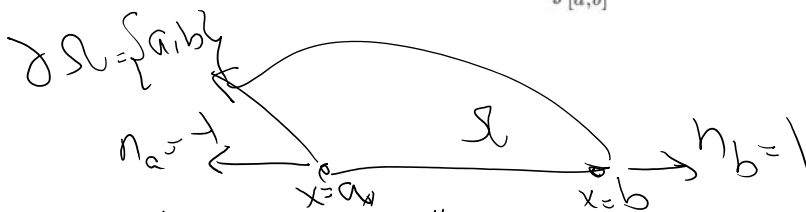
$$\int_{\partial\Omega} \mathbf{F} \cdot \mathbf{n}_i dS = \int_{\Omega} F_{,i} dV$$

$$\int_{\Omega} \frac{\partial F}{\partial x} dV = \int_{\partial\Omega} F n ds$$

$$\int_{\Omega} \nabla \cdot \mathbf{F} dV = \int_{\partial\Omega} \mathbf{F} \cdot \vec{n} ds$$

This is the generalization of the 1D version:

$$1. F(b) + (-1) \cdot F(a) = F(b) - F(a) = \int_{[a,b]} F'(x) dx$$



$$\int_{\partial\Omega} F'(x) dx = \int_{\partial\Omega} F n ds = F(a) n_a + F(b) n_b$$

$$= F(b) \cdot 1 + F(a) \cdot (-1)$$

$$= F(b) - F(a)$$

$$= F(b) - F(a)$$

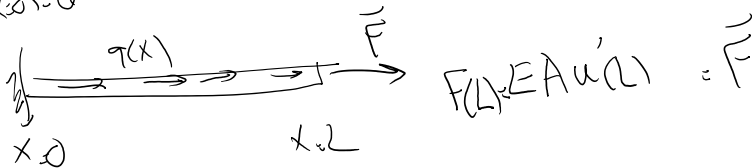
Balance law => Strong Form (PDE)
Can we solve the PDE now?

No

We need to "close" the system by adding more equations:

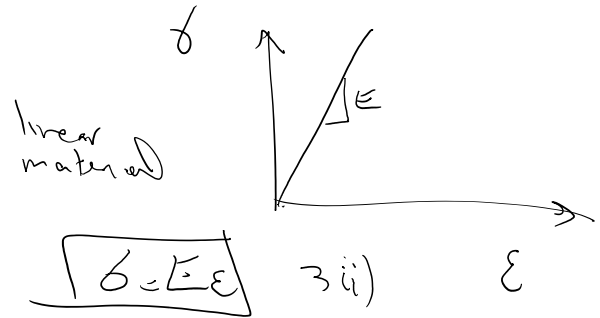
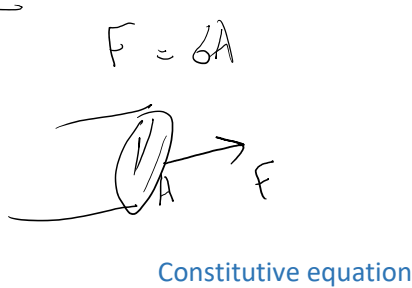
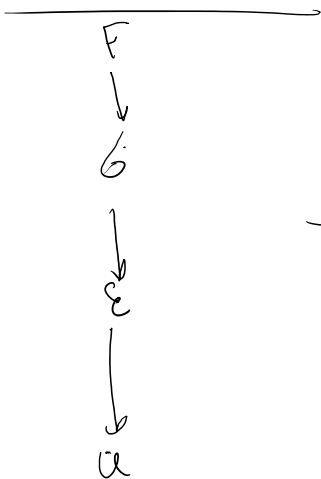
- Constitutive equations
- Compatibility equations
- ...

Example 1: Elastostatic
 $u(x=0) = \bar{u}$



DF

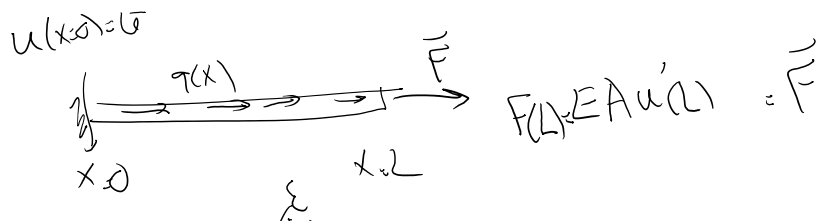
$$\frac{dF(x)}{dx} + q(x) = 0 \quad (3i)$$



Compatibility equation

$$\epsilon = \frac{du}{dx} \quad (3iii)$$

3i - 3iii



$$\frac{dF}{dx} = \frac{dAb}{dx} = \frac{dAE\epsilon}{dx} = \frac{dAE \left(\frac{du}{dx} \right)}{dx}$$

$$\frac{dF}{dx} + q = 0 \implies \textcircled{4} \left[\frac{d}{dx} \left(EA \frac{du}{dx} \right) + q = 0 \right] \text{ Differential Eq Strong form}$$

$$\left(\quad \right)' = \frac{d}{dx}$$

$$\left[(EAu')' + q = 0 \right]$$

order of differential eq $M = 2$

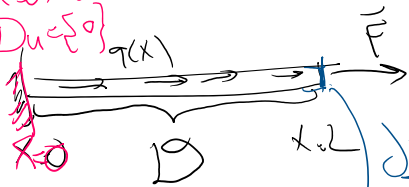
We also need to add the boundary conditions (BCs)

$$\text{half of order of DE } m = \frac{M}{2} = 1$$

BCs

Dirichlet (Essential) BC: value of the function is specified

$$u(x=0) = \bar{u} \text{ order } 0(u)$$



$$F(L) = EAu'(L) = \bar{F}$$

Neumann (Natural) BC

$$F(L) = EAu'(L) = \bar{F} \text{ order } 1$$

Neumann

$$M = 2 \quad (EAu')' + q = 0$$

BCs lower order than M

0	$u(x_0) = \bar{u}$
1	$F = EAu'(x_0) = \bar{F}$

$$u(0) = \bar{u}_0 = 0.01$$

$$u = \bar{u}_1 = 0.02$$




2 Dirichlet

2 Neumann

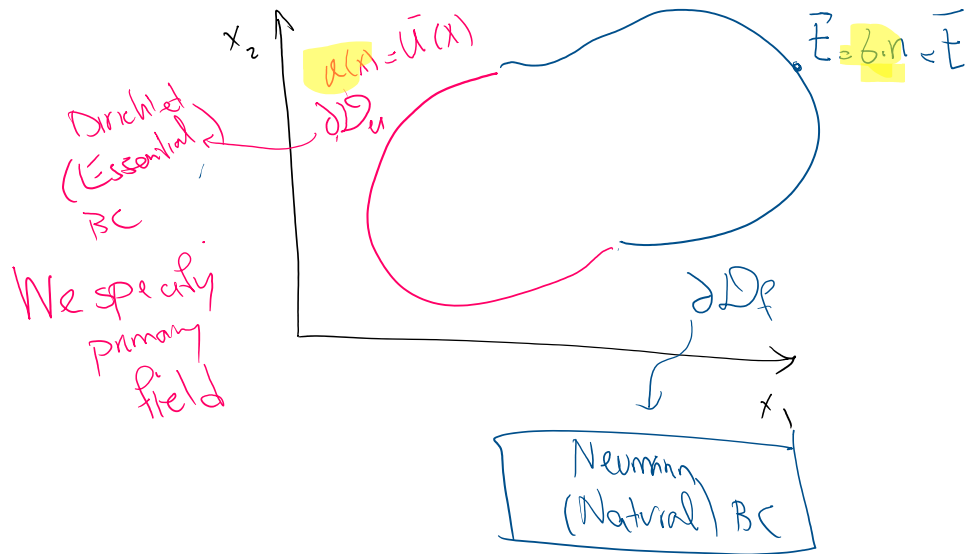


2 Neumann
 Not suitable for static



2D/3D elastostatics

We already took care of BCs



Strong form

$$\nabla \cdot \sigma + \rho b = 0$$

2D

$$\sigma = \begin{pmatrix} \sigma_{11} & \sigma_{12} \\ \sigma_{12} & \sigma_{22} \end{pmatrix}$$

Symmetric matrix $\sigma_{21} = \sigma_{12}$

$$\nabla \cdot \sigma + \rho b = 0$$

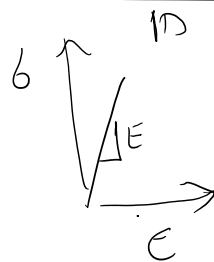
3 unknown stresses

2 equations

$$\begin{cases} \sigma_{11,1} + \sigma_{12,2} + \rho b_1 = 0 \\ \sigma_{12,1} + \sigma_{22,2} + \rho b_2 = 0 \end{cases} \quad (\sigma_{ij,j} + \rho b_i = 0)$$

We lack eqns

Constitutive equation (Linear elastic solid)



$$\sigma = E \epsilon$$



$$\sigma = C \epsilon$$

elasticity stiffness

2D

$$\begin{pmatrix} \sigma_{11} & \sigma_{12} \\ \sigma_{12} & \sigma_{22} \end{pmatrix} = \begin{pmatrix} E_{11} & E_{12} \\ E_{12} & E_{22} \end{pmatrix} \begin{pmatrix} \epsilon_{11} & \epsilon_{12} \\ \epsilon_{12} & \epsilon_{22} \end{pmatrix}$$

$$\begin{bmatrix} \sigma_{11} & \sigma_{12} \\ \sigma_{12} & \sigma_{22} \end{bmatrix}$$

$$\begin{bmatrix} \epsilon_{11} & \epsilon_{12} \\ \epsilon_{12} & \epsilon_{22} \end{bmatrix}$$

elasticity stiffness

4 indices

$$\sigma_{ij} = \sum_{k=1}^2 \sum_{l=1}^2 C_{ijkl} \epsilon_{kl}$$

16 values

$$C_{ijkl}$$

1 " " "

to " " "

2 " " "

Side Note there is an easier way to write:

$\sigma \rightarrow \delta$ relation (Voigt notation)

$$\delta = \begin{bmatrix} \sigma_{11} & \sigma_{12} \\ \sigma_{12} & \sigma_{22} \end{bmatrix} = \sigma \quad \text{where} \quad \epsilon = \begin{bmatrix} \epsilon_{11} & \epsilon_{12} \\ \epsilon_{12} & \epsilon_{22} \end{bmatrix}$$

$$S = \begin{bmatrix} \sigma_{11} \\ \sigma_{22} \\ \sigma_{12} \end{bmatrix}$$

Voigt stress

$$= \underset{3 \times 3}{\tilde{C}} \gamma$$

Voigt stiffness

$$\gamma = \begin{bmatrix} \epsilon_{11} \\ \epsilon_{22} \\ 2\epsilon_{12} \end{bmatrix}$$

Voigt (Engineering) strain

$$\begin{bmatrix} \sigma_{11} \\ \sigma_{22} \\ \sigma_{12} \end{bmatrix} = \begin{bmatrix} \tilde{C}_{11} & \tilde{C}_{12} & \tilde{C}_{13} \\ \tilde{C}_{12} & \tilde{C}_{22} & \tilde{C}_{23} \\ \tilde{C}_{13} & \tilde{C}_{23} & \tilde{C}_{33} \end{bmatrix} \begin{bmatrix} \epsilon_{11} \\ \epsilon_{22} \\ 2\epsilon_{12} \end{bmatrix}$$

symmetric

End of side note

$$\nabla \cdot \sigma + \rho b = 0 \quad \text{eq1} \quad \left\{ \begin{array}{l} \sigma_{11,1} + \sigma_{12,2} + \rho b_1 = 0 \\ \sigma_{12,1} + \sigma_{22,2} + \rho b_2 = 0 \end{array} \right.$$

$$\sigma = \begin{bmatrix} \sigma_{11} & \sigma_{12} \\ \sigma_{12} & \sigma_{22} \end{bmatrix} \rightarrow 3 \text{ unknowns}$$

$$\sigma_{11} = \tilde{C}_{11} \epsilon_{11} + \tilde{C}_{12} \epsilon_{22} + \tilde{C}_{13} \epsilon_{12}$$

3rd added

$$\sigma = C \epsilon$$

3rd added

$$\begin{cases} \sigma_{11} = \tilde{C}_{11} \epsilon_{11} + \tilde{C}_{12} \epsilon_{22} + \tilde{C}_{13} \epsilon_{12} \\ \sigma_{22} = \dots \\ \sigma_{12} = \dots \end{cases}$$

$$\epsilon = \begin{bmatrix} \epsilon_{11} & \epsilon_{12} \\ \epsilon_{12} & \epsilon_{22} \end{bmatrix}$$

3 unknowns added

We are still 1 equation behind

Compatibility equation

1D $\epsilon = \frac{du}{dx}$

2D & 3D $\epsilon = \frac{\nabla u + (\nabla u)^t}{2}$ transposed

$b_1 = \frac{\partial f}{\partial x_1}, b_2 = \frac{\partial f}{\partial x_2}$

Expanded form

$$\epsilon = \begin{bmatrix} \epsilon_{11} & \epsilon_{12} \\ \epsilon_{12} & \epsilon_{22} \end{bmatrix} = \begin{bmatrix} u_{1,1} & \frac{u_{1,2} + u_{2,1}}{2} \\ \frac{u_{1,2} + u_{2,1}}{2} & u_{2,2} \end{bmatrix}$$

eq 1) $\epsilon_{11} = u_{1,1}$

eq 2) $\epsilon_{22} = u_{2,2}$

eq 3) $\epsilon_{12} = \frac{(u_{1,2} + u_{2,1})}{2}$

we just add u_1 & u_2

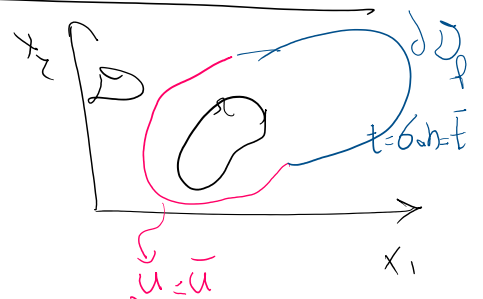
😊 Now # eqns & # unknowns

are the same \implies

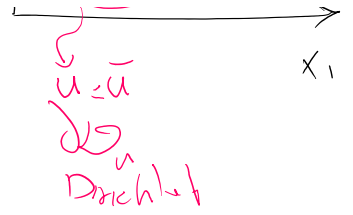
We can solve this

Summary of 2D/3D elasticity

Balance law $\int_{\partial \Omega} \sigma \cdot n \, ds + \int_{\Omega} \rho b \, d\Omega = 0 \implies$



Balance law $\int_{\partial \Omega} \sigma \cdot n \, ds + \int_{\Omega} p b \, d\Omega = 0 \implies$



Strong form $\nabla_x \cdot \sigma + p b = 0$ 2 eqns (2D)

Constitutive eqn

$$\sigma = C \epsilon$$

Compatibility eqn

$$\epsilon = \frac{\nabla u + (\nabla u)^T}{2}$$

Strong form in terms of primary solution field u

$$\nabla \cdot C \epsilon + p b = 0$$

$$\nabla \cdot C \left(\frac{\nabla u + \nabla u^T}{2} \right) + p b = 0$$

form symmetric of $C \implies$

⑧ $\nabla \cdot C \nabla u + p b = 0$ 2D/3D

$\frac{d}{dx} \left(EA \frac{du}{dx} \right) + q = 0$ 1D

HW $-\nabla \cdot q + Q = 0$

$q = -k \nabla T$

- Order of PDE $M = m = 2$ ($m = 1$)

BCs

$M=2$

○	$\forall x \in \partial \Omega_u \quad u(x) = \bar{u}(x)$
1	$\forall x \in \Gamma \quad t(x) = \hat{t}(x) \cdot n(x) = C \nabla u \cdot n = \hat{t}(x)$

v, ..., u

$$\forall x \in \mathcal{D} \quad t(x) = \sigma(x) \cdot n(x) = C \nabla u \cdot n = \bar{t}(x)$$

$$C \varepsilon = C \frac{\nabla u + \kappa y}{2} = C \nabla u$$

order 1 in U

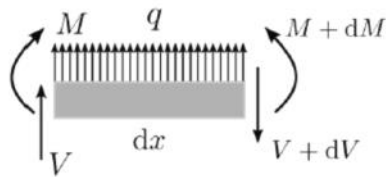
$$t(x) = \sigma(x) \cdot n(x) = \bar{t}(x)$$

Sample Boundary value problems: Euler Bernoulli beam

$\nabla \cdot \mathbf{F} - \mathbf{r} = 0$	Balance law	}	⇒	(28)	
$\mathbf{F} = \begin{bmatrix} M \\ V \end{bmatrix}$	Flux				$\frac{dM}{dx} - V = 0$
$\mathbf{r} = \begin{bmatrix} V \\ q \end{bmatrix}$	Source term				$\frac{dV}{dx} - q = 0$
$M = EI\kappa$	Constitutive equation				$\frac{d^2 EI}{dx^2} \left(\frac{d^2 y}{dx^2} \right) - q = 0$
$\kappa = \frac{d^2 y}{dx^2}$	Kinematic compatibility				

where

- M = Momentum
- V = Shear force
- q = Distributed load
- E = Elastic modulus
- I = Second moment of area
- κ = Curvature
- y = Vertical displacement



Euler Bernoulli beam: BCs

Operator	Sample	elastostatics	operator order
$L_{2m}(u) = r$	$\frac{d^2 EI}{dx^2} \left(\frac{d^2 y}{dx^2} \right) = q$	$\frac{d^2 EI}{dx^2} \left(\frac{d^2 (\cdot)}{dx^2} \right) = q$	$m = 2(M = 4)$
$L_u(u) = \mathbf{n}$	$u = \begin{bmatrix} \theta \\ y \end{bmatrix} = \begin{bmatrix} \frac{dy}{dx} \\ y \end{bmatrix} = \begin{bmatrix} \tilde{\theta} \\ \tilde{y} \end{bmatrix} = \tilde{u}$	$L_u = \begin{bmatrix} \frac{d(\cdot)}{dx} \\ (\cdot) \end{bmatrix}$	$M_u = \begin{bmatrix} 1 \\ 0 \end{bmatrix}$
$L_f(u) = \tilde{\mathbf{f}}$	$\begin{bmatrix} EI \frac{d^2 y}{dx^2} \\ \frac{d}{dx} \left(EI \frac{d^2 y}{dx^2} \right) \end{bmatrix} = \begin{bmatrix} \tilde{M} \\ \tilde{V} \end{bmatrix}$	$L_f = \begin{bmatrix} EI \frac{d^2 (\cdot)}{dx^2} \\ \frac{d}{dx} \left(EI \frac{d^2 (\cdot)}{dx^2} \right) \end{bmatrix}$	$M_f = \begin{bmatrix} 2 \\ 3 \end{bmatrix}$

- One and only one of the pair M (Neumann) and θ (Dirichlet) is enforced at each end of the beam.
- One and only one of the pair V (Neumann) and y (Dirichlet) is enforced at each end of the beam.
- Neumann boundary conditions correspond to the flux terms (M and V).
- Neumann boundary conditions fall in the upper half of derivatives ($[m, 2m - 1] = [2, 3]$).
- Dirichlet boundary conditions fall in the lower half of derivatives ($[0, m - 1] = [0, 1]$).
- There are two boundary conditions at each end point (equal to $m = M/2$).
- $M_u + M_f = M - 1$.

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And slide 34

Read before the class last time