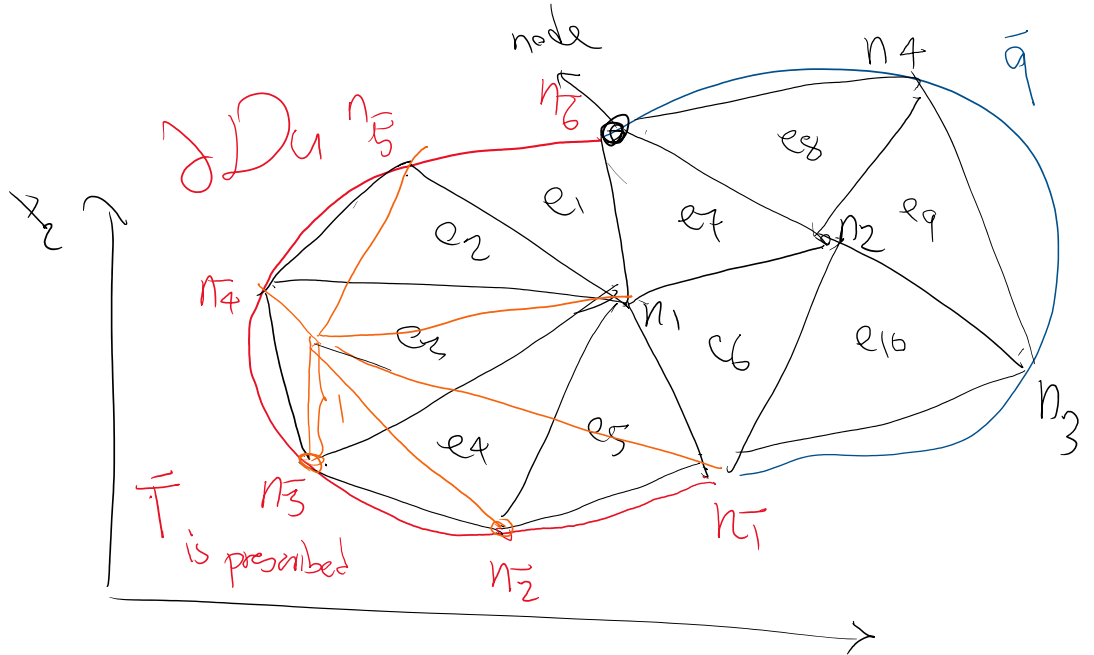
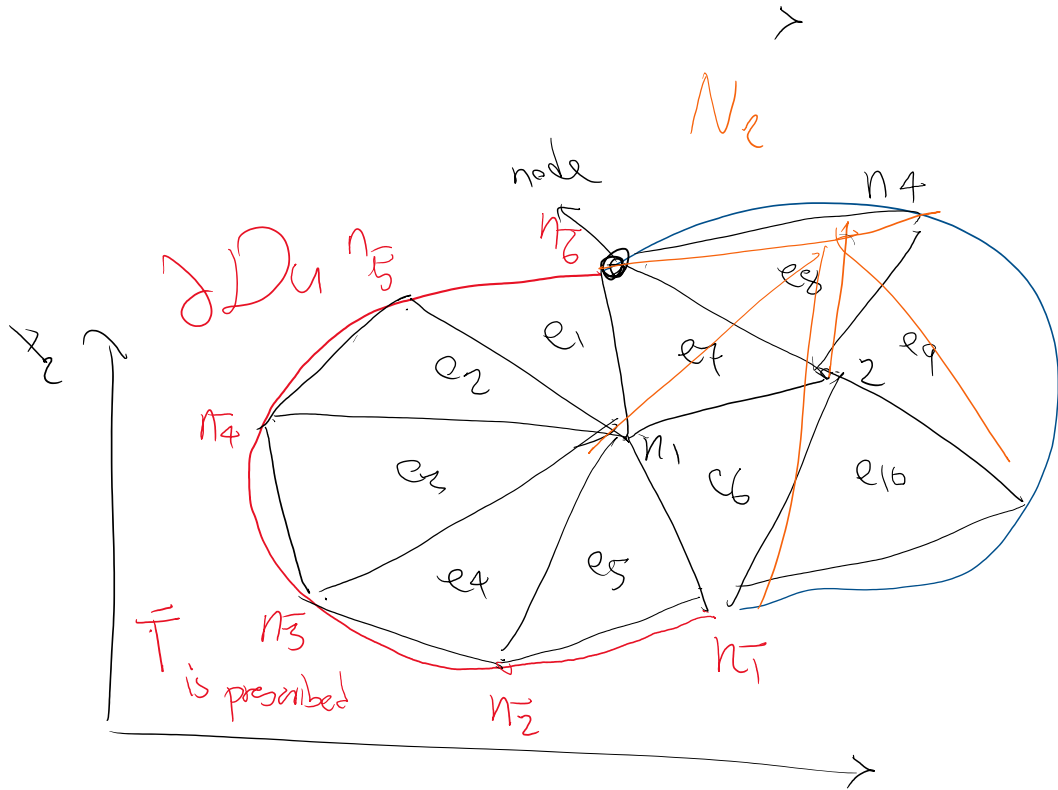


$$n_p = 6$$

$$n_f = 6$$

$$n = n_p + n_f = 12$$



N_1 to N_6 will be used to form ϕ_p

Recall

basis function ϕ_i



FEM shape function N_i

Recall

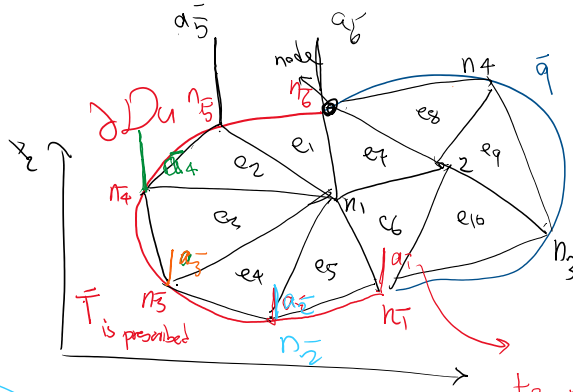
basis functions ϕ_i



shape functions N_i

Dirichlet BC (ϕ_p) using shape functions:

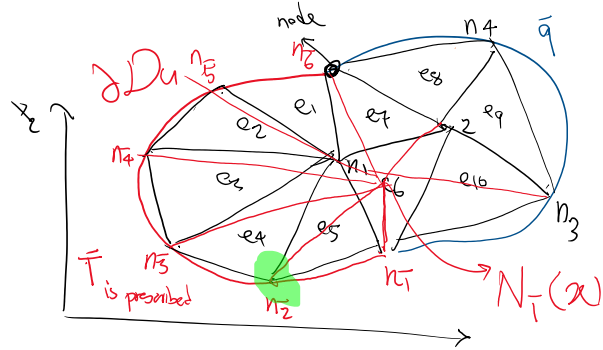
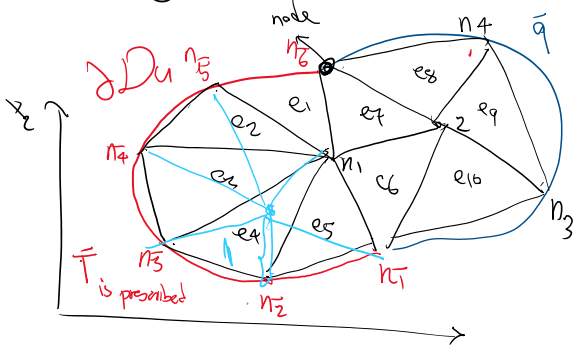
$$\phi_p(x) = \alpha_1 N_1(x) + \alpha_2 N_2(x) + \dots + \alpha_6 N_6(x)$$



temperature at n_1

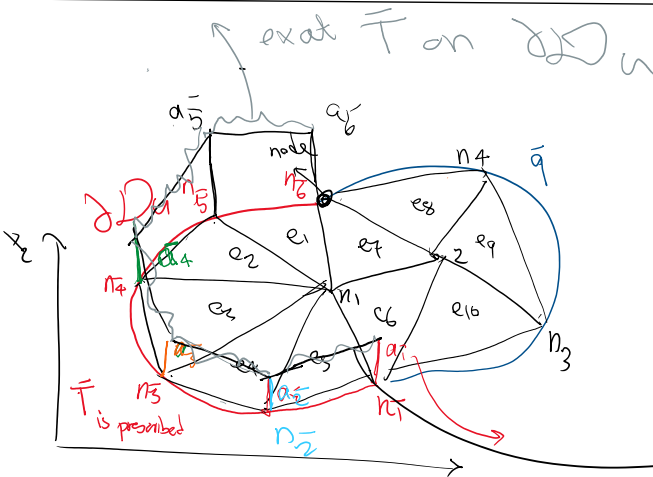
$$\phi_p(n_1) = \alpha_1 N_1(n_1) + \alpha_2 N_2(n_1) + \dots + \alpha_6 N_6(n_1)$$

$\downarrow \quad \quad \quad \downarrow$
0 1 0



$$\phi_p(n_2) = \alpha_2$$

Similarly for $i=1, \dots, 6$ $\phi_p(n_i) = \alpha_i$



we needed

$$\forall x \in \partial \Omega_u \quad \phi_p(x) = \bar{T}(x)$$

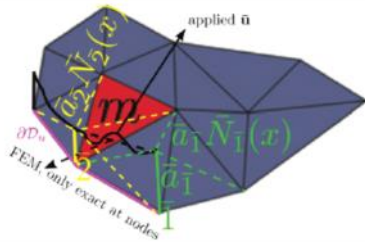
But we only satisfy this condition @ prescribed nodes!

FEM $\phi_p(x)$



1 1 1 1 1

B. Essential Boundary Conditions



$$\begin{aligned}
 n_p &:= \text{number of (prescribed) dof on } \partial D_u \\
 (\cdot) &= \text{decoration for prescribed dofs} \\
 \{\bar{I}_1, \dots, \bar{n}_p\} &= \text{global dofs on } \partial D_u \\
 \bar{a} = [\bar{a}_{\bar{I}_1}, \dots, \bar{a}_{\bar{n}_p}]^T &= \text{vector of prescribed values for these dofs} \\
 \bar{N} = [\bar{N}_{\bar{I}_1}, \dots, \bar{N}_{\bar{n}_p}] &= (\text{row}) \text{ vector of shape functions for these dofs} \\
 \bar{B} = [\bar{B}_{\bar{I}_1}, \dots, \bar{B}_{\bar{n}_p}] &= (\text{row}) \text{ vector of "displacement to strain" for these dofs}
 \end{aligned} \tag{308}$$

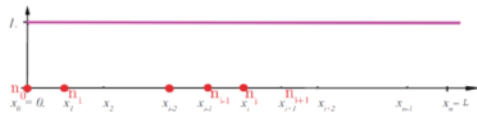
• ϕ_p is formed by,

$$\phi_p = \bar{N} \bar{a} = \sum_{i=1}^{n_p} \bar{a}_{\bar{I}_i} \bar{N}_{\bar{I}_i} \Rightarrow \frac{d\phi_p}{dx} = \bar{B} \bar{a} = \sum_{i=1}^{n_p} \bar{a}_{\bar{I}_i} \bar{B}_{\bar{I}_i} \tag{309}$$

- While this construction guarantees satisfaction of essential boundary conditions at nodes $\bar{I}_1, \dots, \bar{n}_p$, as shown in the figure the intermediate values may not match \bar{u} .

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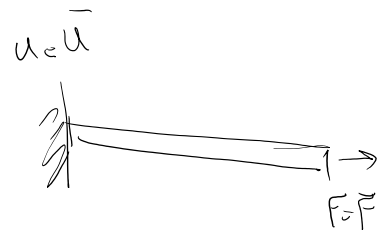
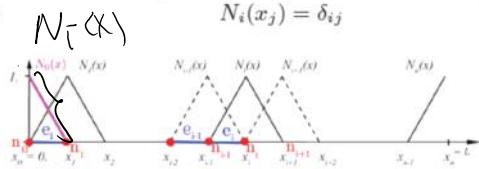
B. Essential Boundary Conditions



- Previously to satisfy the essential conditions for ϕ_p we used,

$$\phi_p = \bar{u} \mathbf{1} \Rightarrow \phi_p(0) = \bar{u}$$

- However, Finite Element Shape functions provide a natural way to construct ϕ_p by using (302):



• We let,

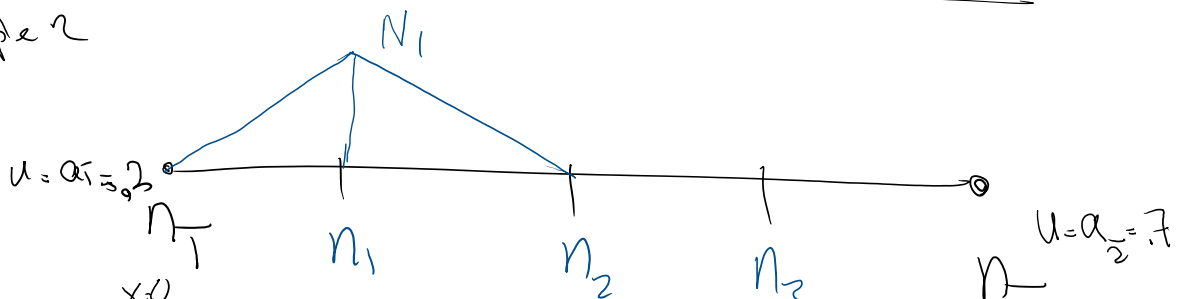
$$\bar{\phi}_p = \bar{u} N_0 \Rightarrow \phi_p(0) = \phi_p(x_0) = \bar{u} N_0(x_0) = \bar{u} \delta_{11} = \bar{u} \tag{307}$$

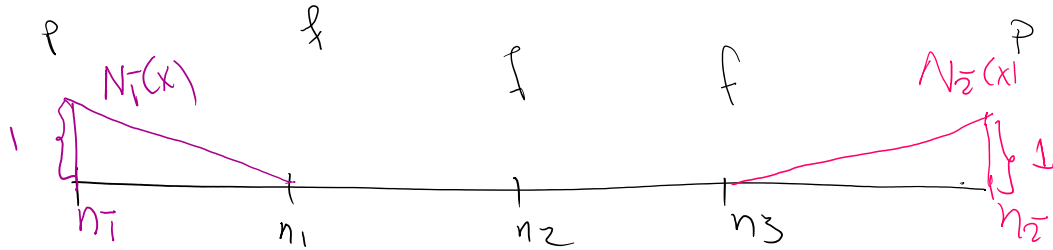
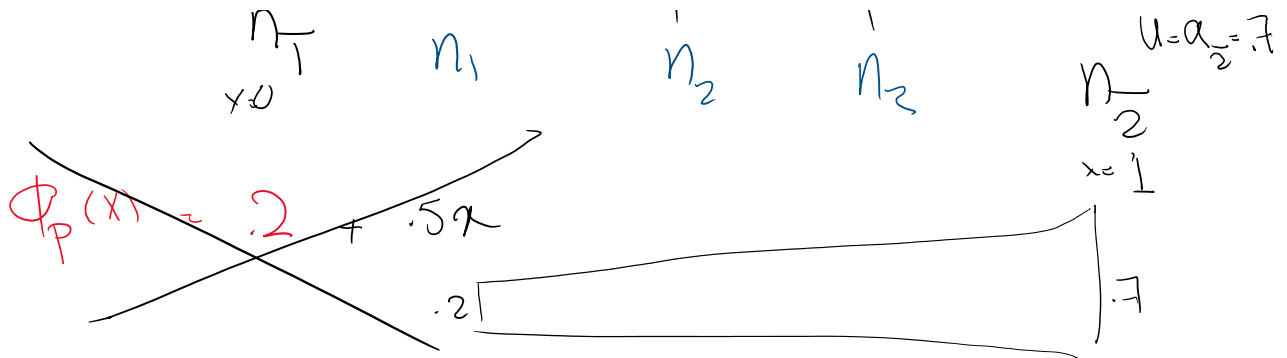
- It is clear that for all the trial functions (i.e., shape functions corresponding to unknowns - $I \in \{N_1, \dots, N_{n_f}\}$), $N_I(0) = N_I(x_0) = \delta_{I0} = 0$. That is, trial functions satisfy homogeneous essential boundary condition.

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$$\phi_p = \underbrace{\bar{a}_{\bar{I}_1}}_{\bar{a}} N_{\bar{I}_1}(x) = \bar{a} N_{\bar{I}_1}(x)$$

Example 2





$$\Phi_p(x) = a_1 N_1(x) + a_2 N_2(x)$$

$$= 0.2 N_1(x) + .7 N_2(x)$$

K_p

same example

$$u^h(x) = \Phi_p(x) + \sum_{i=1}^{n_f} a_i N_i$$

free dof

$$\frac{n_f = 3}{n_p = 2}$$

$$\Phi_p(x) = \sum_{i=1}^{n_p} \alpha_i N_i(x)$$

$$u^h = \underbrace{\sum_{i=1}^{n_p} \alpha_i N_i(x)}_{\Phi_p} + \sum_{i=1}^{n_f} a_i N_i(x)$$

$$= \underbrace{\begin{bmatrix} N_1 & N_2 & \dots & N_{n_p} \end{bmatrix}}_{N_p} \underbrace{\begin{bmatrix} \alpha_1 \\ \vdots \\ \alpha_{n_p} \end{bmatrix}}_{a_p} + \underbrace{\begin{bmatrix} N_1(x) & \dots & N_{n_f}(x) \end{bmatrix}}_{N_f} \underbrace{\begin{bmatrix} a_1 \\ \vdots \\ a_{n_f} \end{bmatrix}}_{a_f}$$

$$= N_p a_p + N_f a_f$$

$$u^h = \underbrace{N_p}_{P_p} a_p + N_f a_f$$

$$N_p = [N_1 \dots N_{n_p}] a_p = \begin{pmatrix} a_1 \\ \vdots \\ a_{n_p} \end{pmatrix}$$

$$N_f = [N_1 \dots N_{n_f}] a_f = \begin{pmatrix} a_1 \\ \vdots \\ a_{n_f} \end{pmatrix}$$

Ex
 $n_p = 2$
 $n_f = 3$

①

$$\epsilon = u^h' = \left(\sum_{i=1}^{n_p} N_i'(x) a_i \right)' + \left(\sum_{i=1}^{n_f} N_i(x) a_i \right)'$$

$$= \sum_{i=1}^{n_p} N_i'(x) a_i + \sum_{i=1}^{n_f} N_i'(x) a_i$$

$$m(u) = B_p a_p + B_f a_f$$

$$B_p = L_m(N_p) = [L_m(N_1), \dots, L_m(N_{n_p})]$$

(for a bar $L_m(\cdot) = (\cdot)'$)
 $B_f = [N_1', \dots, N_{n_f}']$

②

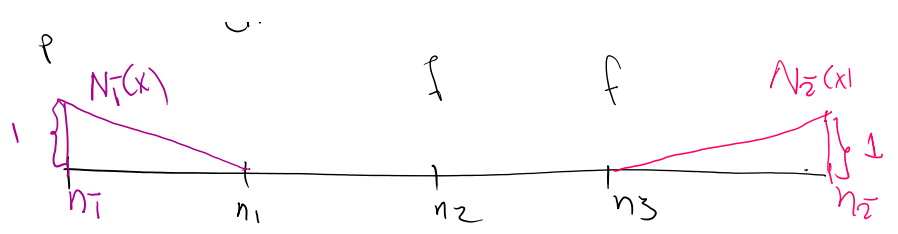
$$B_f = L_m(N_f) = [L_m(N_1), \dots, L_m(N_{n_f})]$$

(for a bar problem)
 $B_f = [N_1', \dots, N_{n_f}']$

for bar $\epsilon(x) = \underbrace{B_p(x) a_p}_{\substack{\text{Displacement} \\ \text{to strain} \\ \text{operator} \\ \text{matrix}(x)}} + \underbrace{B_f(x) a_f}_{\substack{\text{displacement} \\ \text{to strain} \\ \text{operator}}}$

\downarrow nodal disp \downarrow nodal disp





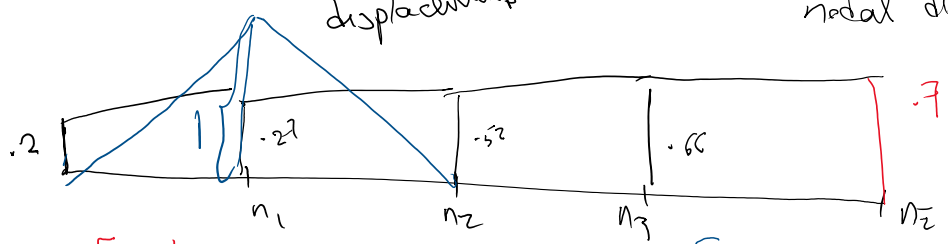
$$a_p = \begin{bmatrix} .2 \\ .7 \end{bmatrix}$$

$$a_f = \begin{bmatrix} P \\ f \\ f \end{bmatrix}$$

FEM soluti: $a_f = \begin{bmatrix} .27 \\ .52 \\ .66 \end{bmatrix}$

Strain $\epsilon(x) = [B_1(x) \ B_2(x)] \begin{bmatrix} .2 \\ .7 \end{bmatrix} + [B_1(x) \ B_2(x) \ B_3(x)] \begin{bmatrix} .27 \\ .52 \\ .66 \end{bmatrix}$

Node displacements a_p nodal displacement a_f



$$u^h = [N_1 \ N_2] \begin{bmatrix} .2 \\ .7 \end{bmatrix} + [N_1(x) \ N_2(x) \ N_3(x)] \begin{bmatrix} .27 \\ .52 \\ .66 \end{bmatrix}$$

$$u^h(n_2) = 0.52 = a_2$$

$$u^h(n_1) = 0.2 = a_1$$

a_i = Solution \odot
 node i
 $N_i(n_j) = \delta_{ij} = \begin{cases} 1 & i=j \\ 0 & i \neq j \end{cases}$

Bar Example: Overview

$n_p = 1$
 $n_f = 4$

step 1: shape functions & B's

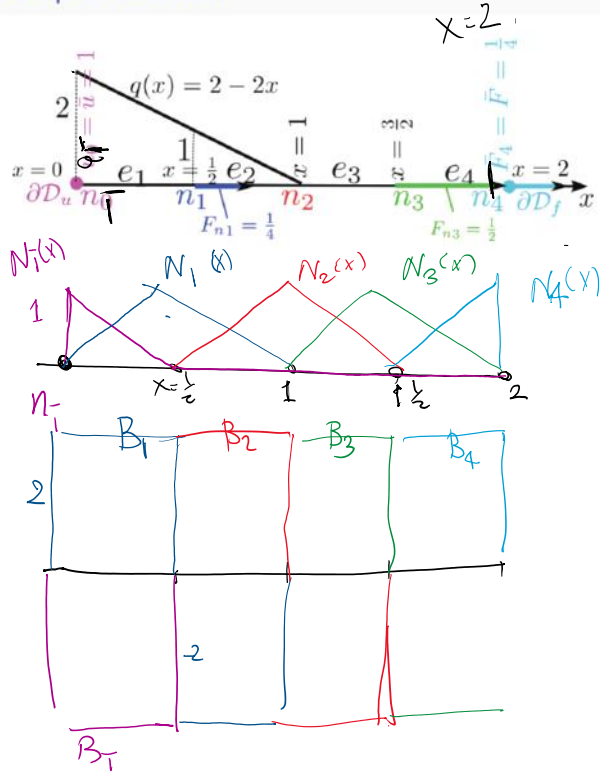
$\bar{p} = \bar{a} = [\alpha_i] = [1]$

$a_f = a = \begin{bmatrix} a_1 \\ a_2 \\ a_3 \\ a_4 \end{bmatrix} = ?$ $B's \quad L_m = C$
 $B = N'$

in general

$K = K_{ff} = \int_{\mathcal{D}} \begin{bmatrix} L_m(\phi_1) \\ \vdots \\ L_m(\phi_n) \end{bmatrix} D [L_m(\phi_1) \dots L_m(\phi_n)] dv$

FEM $\phi \rightarrow N$
 $L_m(\phi) \rightarrow B$
 $n \rightarrow n_f$



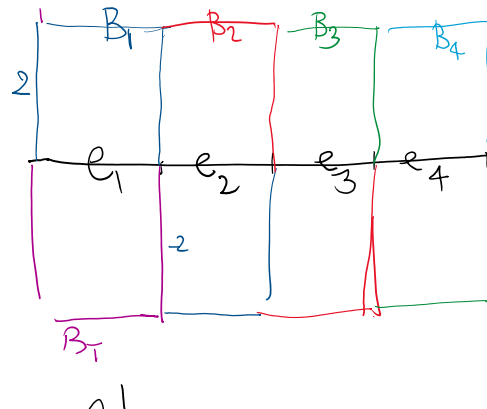
(3)

$$K = K_{ff} = \int_{\mathcal{D}} \begin{bmatrix} B_1 \\ \vdots \\ B_{n_f} \end{bmatrix} D [B_1 \dots B_{n_f}] dv$$

$$= \int_{\mathcal{D}} B_f^t D B_f dv$$

$$K = \int_0^2 \begin{bmatrix} B_1 \\ B_2 \\ B_3 \\ B_4 \end{bmatrix} D [B_1 \ B_2 \ B_3 \ B_4] dx$$

$$K_{11} = \int_0^2 B_1 B_1 dx = \int_0^{1/2} B_1 B_1 dx + \int_{1/2}^1 B_2 B_2 dx + \dots$$



$$K_{11} = \int_D B_1 B_1 dx = \int_{e_1} B_1 B_1 dx + \int_{e_2} B_1 B_1 dx$$

$$\rightarrow \int_{e_3} B_1 B_1 dx + \int_{e_4} B_1 B_1 dx = \int_{x=0}^{.5} (2)(2) dx + \int_{.5}^1 (-2)(-2) dx + 0 + 0 =$$

$$.5 \times 2 \times 2 + .5 \times (-2) \times (-2) = \underline{4}$$

$$K_{12} = \int_0^2 B_1 B_2 dx = \int_{e_2} B_1 B_2 dx = \int_{.5}^1 (-2)(2) dx = .5 \times (-2) \times (2) = \underline{-2}$$

$$K = \begin{bmatrix} 4 & -2 \\ -2 & 4 & -2 \\ & -2 & 4 & -2 \\ & & -2 & 2 \end{bmatrix} \quad \text{④}$$

stiffness
for the
test problem

Recall from last time

$$F_D = c A (\Phi^t, \Phi_p) = c A \left(\begin{bmatrix} \Phi_1 \\ \vdots \\ \Phi_{n_p} \end{bmatrix}, \Phi_p \right) \quad \text{general}$$

N_{n_p}

$$F_D = \int_D L_m \begin{bmatrix} N_1 \\ \vdots \\ N_{n_p} \end{bmatrix} D L_m(\Phi_p) dV \quad \text{FEM}$$

$$\Phi_p = N_p a_p = [N_{\bar{1}} \dots N_{\bar{n}_p}] \begin{bmatrix} a_{\bar{1}} \\ \vdots \\ a_{\bar{n}_p} \end{bmatrix}$$

$$\Gamma \quad \left(\int N_1 \right) \quad \dots \quad \left(\int a_{\bar{1}} \right)$$

$$F_D = \int_D \underbrace{L_m \begin{bmatrix} N_1 \\ \vdots \\ N_{np} \end{bmatrix}}_{B_f^t} D \underbrace{L_m (N_1, \dots, N_{np})}_{B_p} \begin{bmatrix} a_r \\ \vdots \\ a_{\bar{n}_p} \end{bmatrix}$$

$$F_D = \int_D (B_f^t D B_p) a_p$$

domain ← D

$$F_D = K_{fp} a_p \quad (5)$$

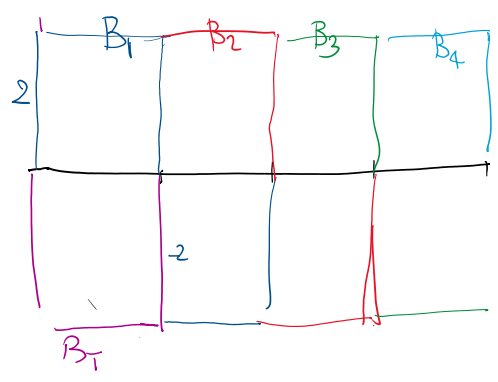
Recall

$$K = K_{ff} = \int_D B_f^t D B_f dv$$

$$F_D = K_{fp} a_p$$

$$K_{fp} = \int_0^2 \begin{bmatrix} B_1 \\ B_2 \\ B_3 \\ B_4 \end{bmatrix} D [B_i] dx$$

EA=1



$$K_{fp} = \begin{bmatrix} \int_0^2 B_1 B_1 dx \\ 0 \\ 0 \\ 0 \end{bmatrix} = \begin{bmatrix} .5 \times (-2) (2) \\ 0 \\ 0 \\ 0 \end{bmatrix} = \begin{bmatrix} -2 \\ 0 \\ 0 \\ 0 \end{bmatrix}$$

$$a_p = a_1 = [1]$$

$$F_D = K_{fp} a_p = \begin{bmatrix} -2 \\ 0 \\ 0 \\ 0 \end{bmatrix} [1] = \begin{bmatrix} -2 \\ 0 \\ 0 \\ 0 \end{bmatrix} \quad (6)$$