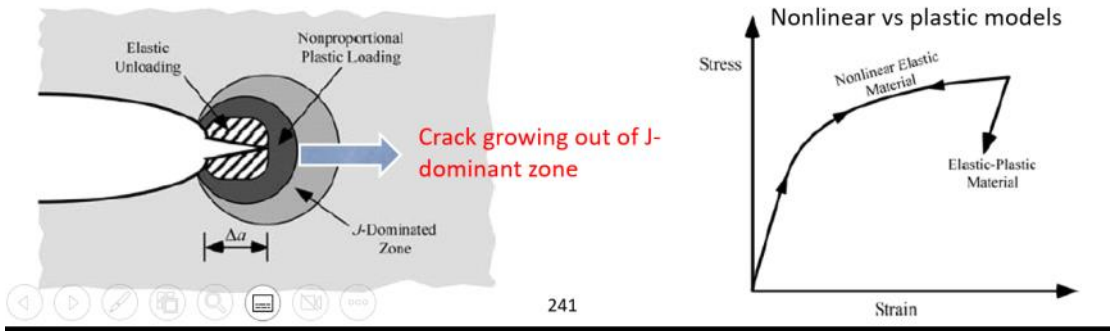


When PFM models will fail?

- When we must use finite deformation theory OR there is a significant plastic unloading

## LSY: When a single parameter (G, K, J, CTOD) is not enough?

- Under considerable plastic deformation and crack propagation when unloading and non-proportional zones grow out of J dominant zone with crack propagation. Reasons are:
  - Unloading: In J integral analysis plastic model was replaced by a nonlinear solid
  - Single-parameter identification not valid since various stress components increase at different rates



What should be done when LY condition is encountered?

- Advanced computational method with appropriate model assumptions (recommended)
- Extend previous models by adding more parameters (K + T stress) or (J + Q)

## LSY: When a single parameter (G, K, J, CTOD) is not enough? T stress

$$\sigma_{ij} = \frac{K_I}{\sqrt{2\pi r}} f_{ij}(\theta) + \begin{bmatrix} T & 0 & 0 \\ 0 & 0 & 0 \\ 0 & 0 & \nu T \end{bmatrix}$$

plane strain

$$\sigma_{xx} = \frac{K_I}{\sqrt{2\pi r}} f_{xx}(\theta) + T$$

T stress

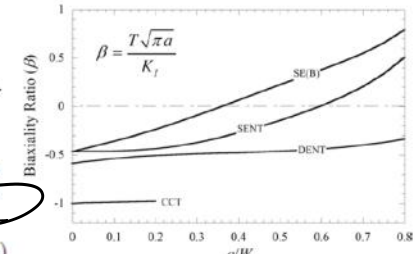
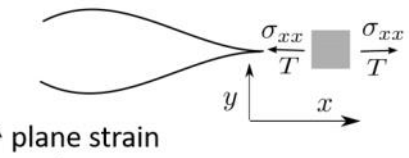
# CRACK PROPAGATION THROUGH PLANE STRESS

• Higher order terms in stress expansion:

- **T stress** (linear analysis)

- \* Constant  $\sigma_{xx}$  in LEFM expansion
- \* Nondimensional biaxiality ratio:  $\beta = \frac{T\sqrt{\pi a}}{K_I}$
- \* Example  $\beta = -1$  for mode-I crack in infinite domain.
- \* **T stress redistributes plastic stress**
- \*  $\beta(T)$  depend on particular geometry/loading configuration
- \* Effect of  $T(\beta)$  on toughness:

$$\sigma_{ij} = \frac{K_I}{\sqrt{2\pi r}} f_{ij}(\theta) + \begin{bmatrix} T & 0 & 0 \\ 0 & 0 & 0 \\ 0 & 0 & \nu T \end{bmatrix}$$



High (+) T  $\Rightarrow$  Constrained (triaxial) stress  $\Rightarrow$  Toughness  $\searrow$  Ductility  $\searrow$

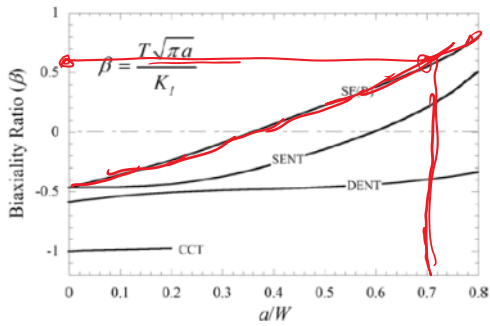
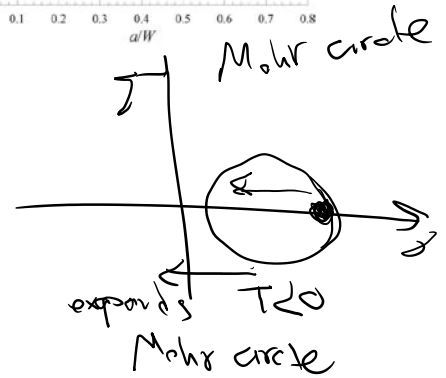
Low (-) T  $\Rightarrow$  Lose constraint  $\Rightarrow$  Toughness  $\nearrow$  Ductility  $\nearrow$

\* T stress also influences crack path stability (particularly in dynamic fracture)

even compress

$$\beta = \frac{T\sqrt{\pi a}}{K_I}$$

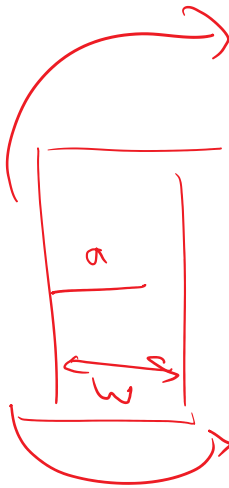
biaxiality ratio



$\beta = .6$

$$T = .6 \frac{K_I}{\sqrt{\pi a}} > 0$$

more brittle fracture than uncracked infinite domain

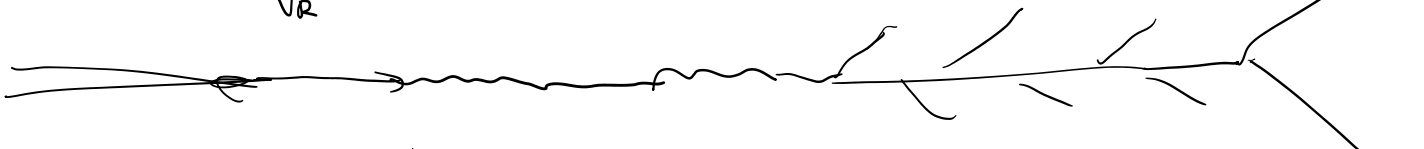
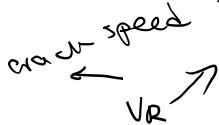


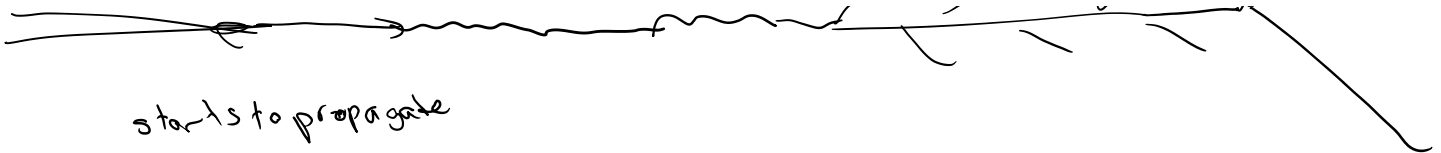
$$\frac{a}{w} = .7$$

max compressive T we can get

maximum ductility

The effect of T stress in dynamic fracture





starts to propagate

$T \text{ stress} > 0$  stabilizes the crack path



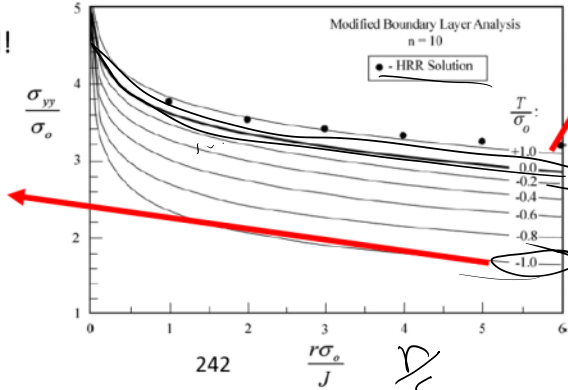
T stress  
 -  $T < 0$  -> more ductile fracture / higher toughness  
 - Higher tensile T stabilizes dynamic crack path

\* T stress also influences crack path stability (particularly in dynamic fracture)

$a/W$

Plastic analysis:  $\sigma_{yy}$  redistributed!  
 Kirk, Dodds, Anderson

High negative T stress:  
 - Decreases  $\sigma_{yy}$   
 - Decreases triaxiality



Positive T stress:  
 - Slightly Increases  $\sigma_{yy}$  and increase triaxiality

$\frac{T}{\sigma_o} = -1$

$\frac{r\sigma_o}{J} \rightarrow$  CTOD

# LSY: When a single parameter (G, K, J, CTOD) is not enough? J-Q theory

- **Q parameter (J-Q theory)** Valid for nonlinear analysis

\* Added as a hydrostatic shift in front of crack to (HRR) stress fields

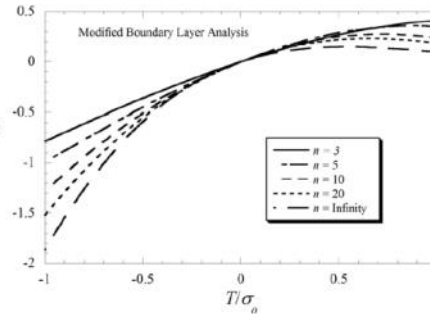
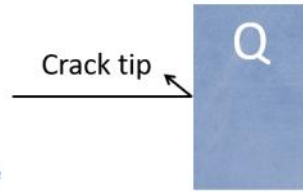
$$\sigma_{ij} \approx (\sigma_{ij})_{T=0} + Q\sigma_0\delta_{ij} \quad \left(|\theta| \leq \frac{\pi}{2}\right)$$

\* Similar to  $T$  positive  $Q$  increases triaxiality and reduces fracture resistance

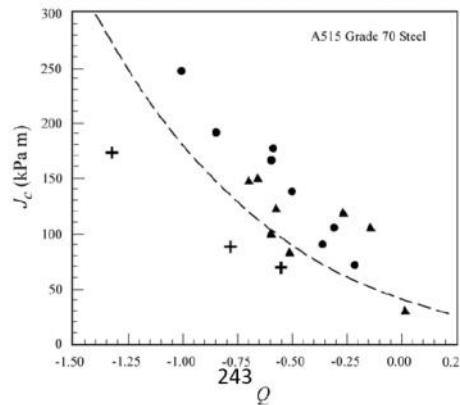
$$J_c = J_c(Q)$$

High (+)  $Q \Rightarrow$  Constrained (triaxial) stress  $\Rightarrow$  Toughness  $\searrow$  Ductility  $\searrow$   
 Low (-)  $Q \Rightarrow$  Lose constraint  $\Rightarrow$  Toughness  $\nearrow$  Ductility  $\nearrow$   $Q$

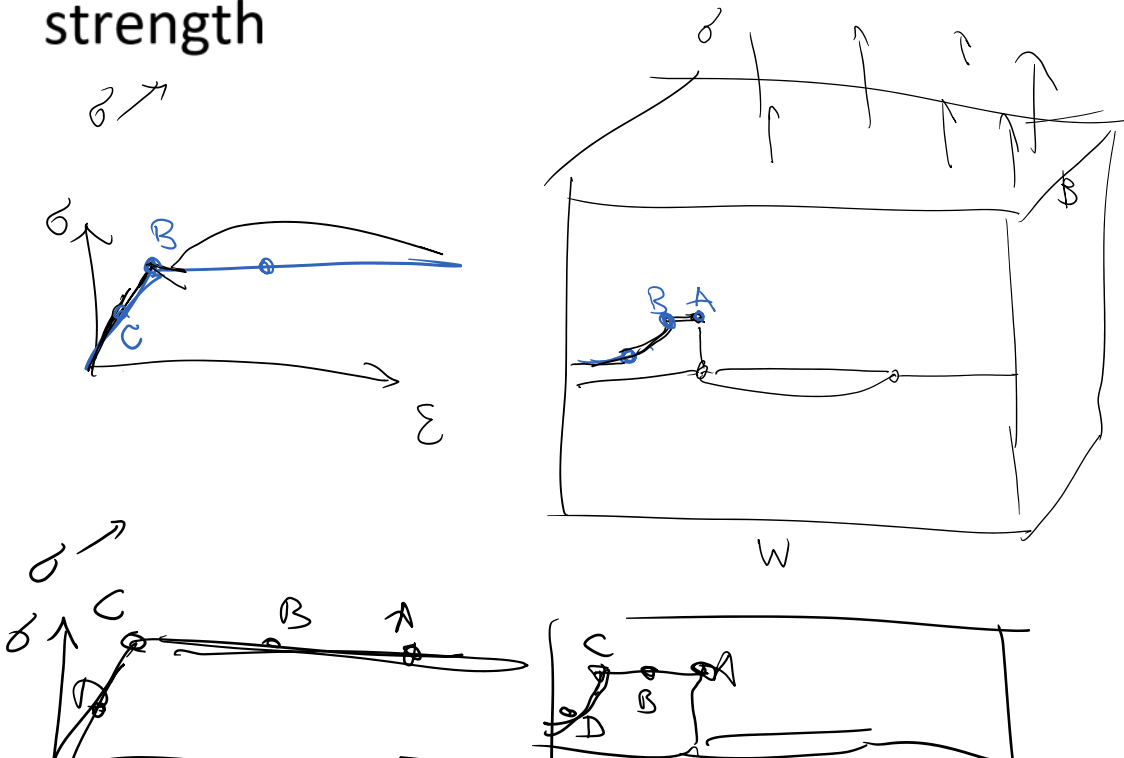
- **More number of parameters:** With extensive deformation two-parameter models such as  $K, T$  or  $J, Q$  eventually break.

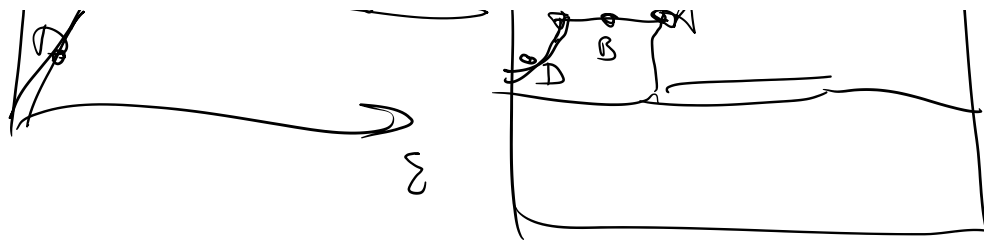


$n$ : strain hardening in HRR analysis



## 5.3. 7. Fracture mechanics versus material (plastic strength)

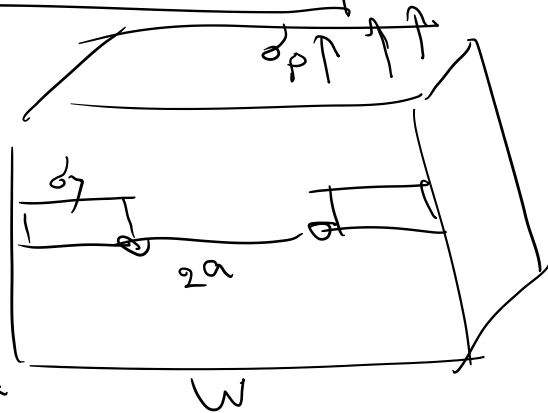




Full plastic load capacity

$$\sigma_p BW = \sigma_y (W-2a) B$$

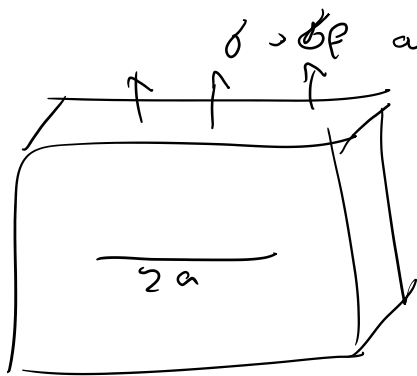
maximum load for plastic failure



What about fracture limit of this

$$\sigma_p = \left( \frac{W-2a}{W} \right) \sigma_y \quad (1)$$

plastic failure



$\sigma \rightarrow \sigma_p$  when fracture criterion is satisfied

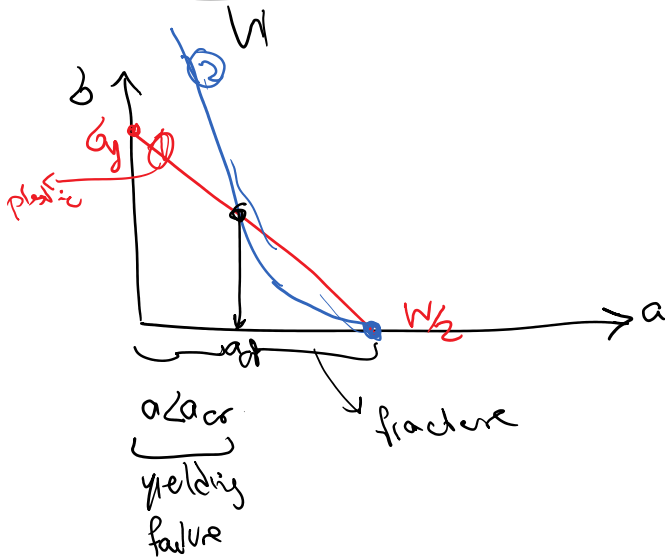
$$K_I = f\left(\frac{a}{W}\right) \sigma \sqrt{\pi a}$$

$$K_{IC} = f\left(\frac{a}{W}\right) \sigma_p \sqrt{\pi a}$$

$\sigma \rightarrow \sigma_p$   
 $K_I \rightarrow K_{IC}$

$$\sigma_p = \frac{K_{IC}}{f\left(\frac{a}{W}\right) \sqrt{\pi a}} \quad (2)$$

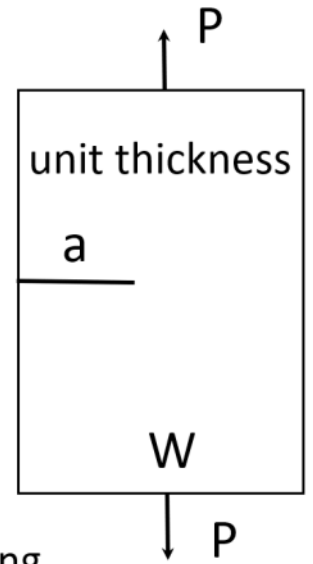
fracture



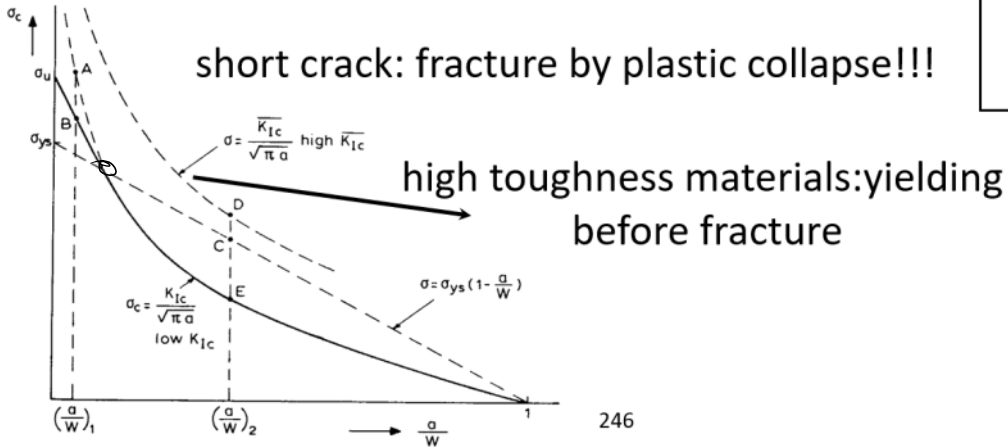
$$\sigma_{net} = \frac{P}{W - a} = \sigma \frac{W}{W - a}$$

(cracked section)

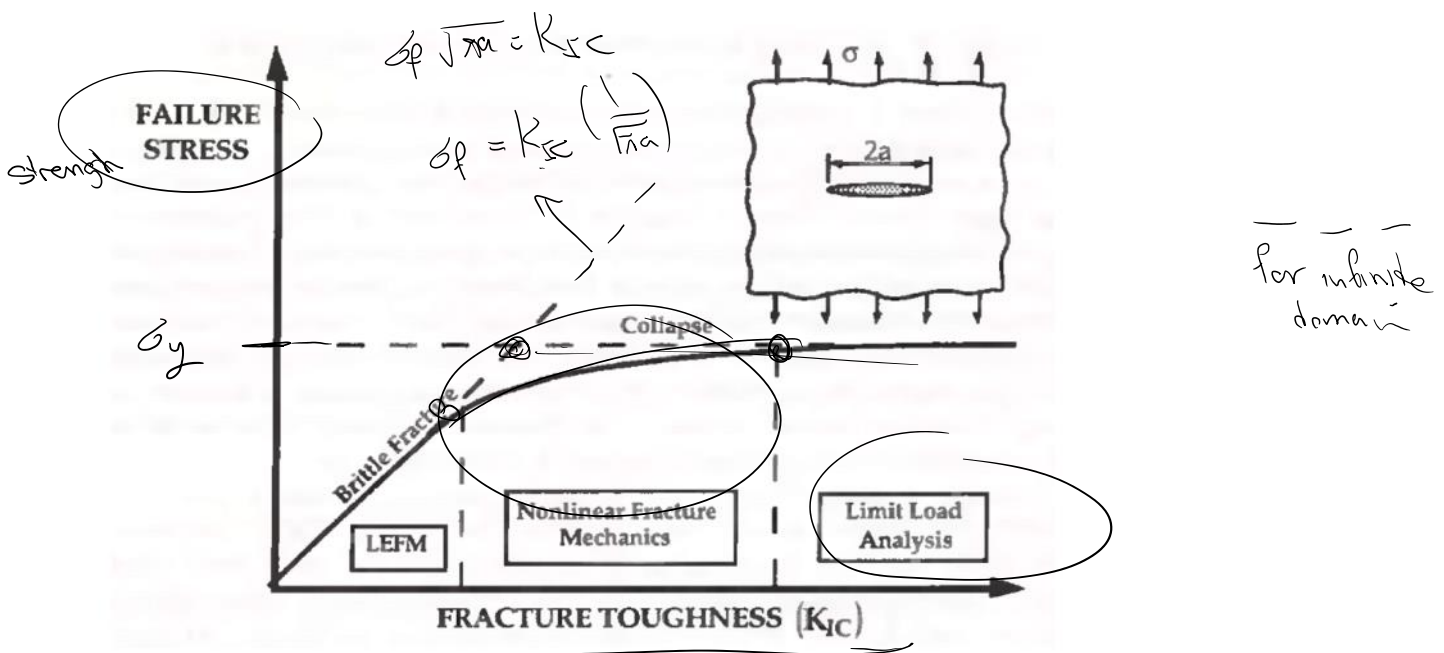
$$\sigma = \frac{P}{W}$$



Yield:  $\sigma \frac{W}{W - a} = \sigma_{ys} \longrightarrow \sigma = \sigma_{ys} \left(1 - \frac{a}{W}\right)$



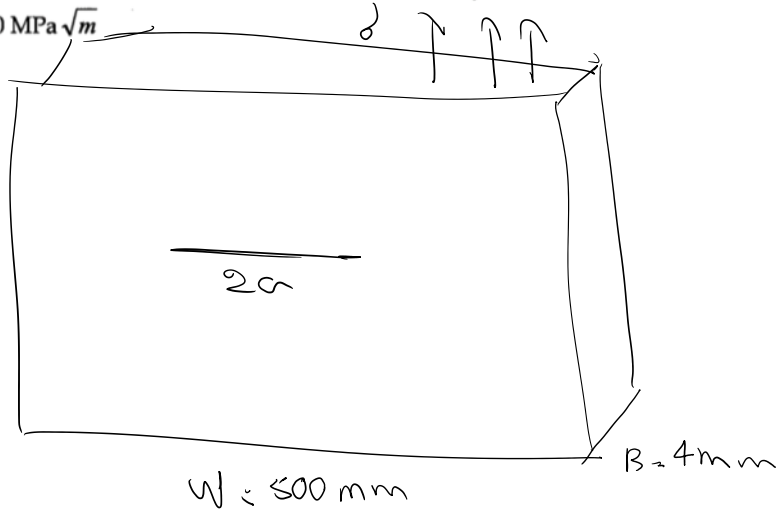
# Governing fracture mechanism and fracture toughness



# Example

**Example 4.11** Estimate the failure load under uniaxial tension for a centre-cracked panel of aluminium alloy of width  $W=500$  mm, and thickness  $B=4$  mm, for the following values of crack length  $2a = 20$  mm and  $2a = 100$  mm. Yield stress  $\sigma_y = 350$

MPa and fracture toughness  $K_{Ic} = 70 \text{ MPa}\sqrt{\text{m}}$



$$\sigma_y = 350 \text{ MPa}$$

$$K_{Ic} = 70 \text{ MPa}\sqrt{\text{m}}$$

1)  $2a = 20 \text{ mm}$

2)  $2a = 100 \text{ mm}$

what is the ultimate load?

①  $2a = 20 \text{ mm}$   $a = 10 \text{ mm}$

yielding  $\sigma_y (W - 2a) B = \sigma_p W B$

$$\sigma_p = \sigma_y \frac{(W - 2a)}{W} = 350 \times \frac{500 - 20}{500} \Rightarrow$$

$$F_p = \sigma_p B W = 672 \text{ kN}$$

Fracture  $\sigma_f = \frac{K_{Ic}}{f(a/W) \sqrt{\pi a}} = \frac{70 \text{ MPa}\sqrt{\text{m}}}{\sqrt{\sec(\frac{2a}{W})} \sqrt{\pi \times (10 \times 10^{-3} \text{ m})}}$

$$F_p = \sigma_f B W = 790 \text{ kN}$$

plastic failure

②  $F_p = 560 \text{ kN}$

$$F_f = 172.2 \text{ kN}$$

fractures first



# 6.1 Fracture mechanics in Finite Element Methods (FEM)

6.1.1. Introduction to Finite Element method

6.1.2. Singular stress finite elements

6.1.3. Extraction of K (SIF), G

6.1.4. J integral

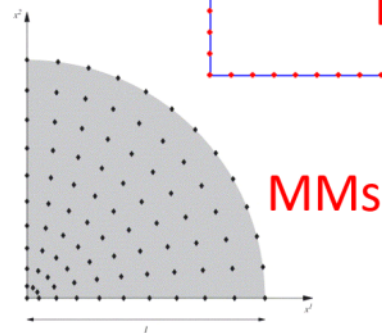
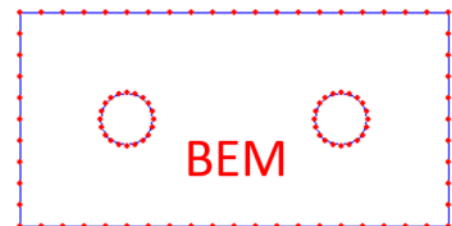
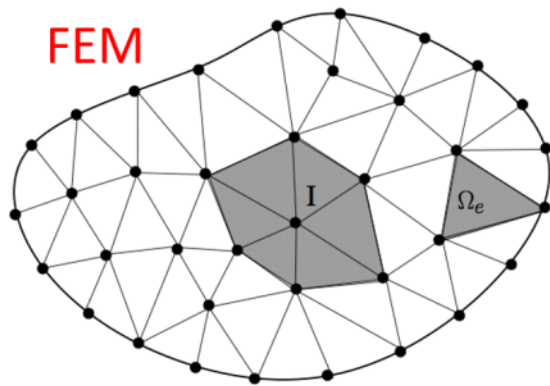
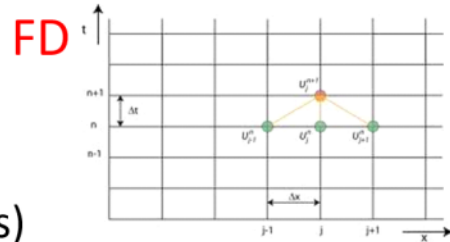
6.1.5. Finite Element mesh design for fracture mechanics

6.1.6. Computational crack growth

6.1.7. Extended Finite Element Method (XFEM)

## Numerical methods to solve PDEs

- Finite Difference (FD) & Finite Volume (FV) methods
- FEM (Finite Element Method)
- BEM (Boundary Element Method)
- MMs (Meshless/Meshfree methods)



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# Fracture models

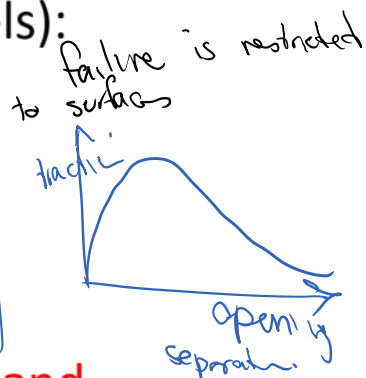
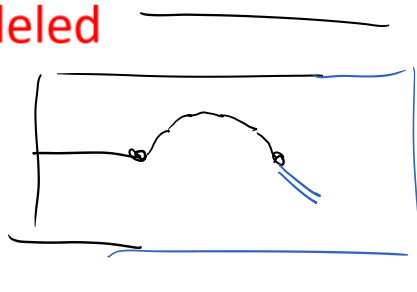
- Discrete crack models (discontinuous models):

Cracks are explicitly modeled

- LEFM

- EPFM

- Cohesive zone models



- Continuous models: Effect of (micro)cracks and voids are incorporated in bulk damage

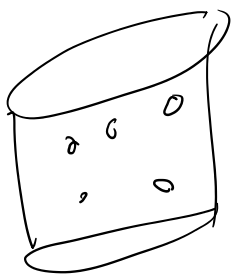
- Continuum damage models

- Phase field models

- Peridynamic models: Material is modeled as a set of particles

Bulk models

Continuum / bulk damage



$$\sigma = \frac{C \epsilon}{(1-D)}$$

$$\sigma = (1-D) C \epsilon$$

$D=1$  fully damaged

phase field model:

Regularizes LEFM (very different

starting point than bulk damage)

$$\sigma = \frac{\omega(d)}{(1-D)} C \epsilon$$

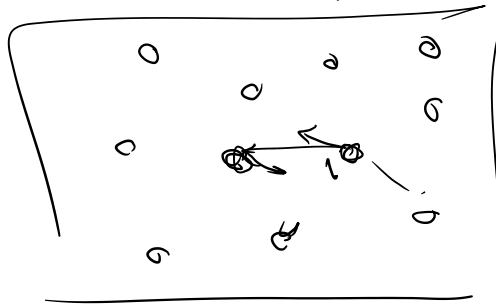
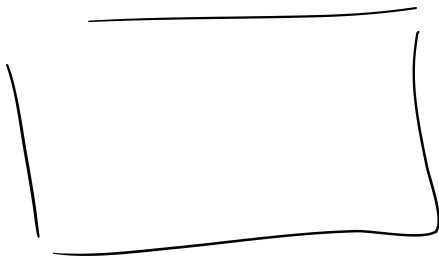
(1-D)

or other forms

Peridynamics similar to Molecular dynamic

o → o / new models

Fluid dynamics similar to molecular dynamic

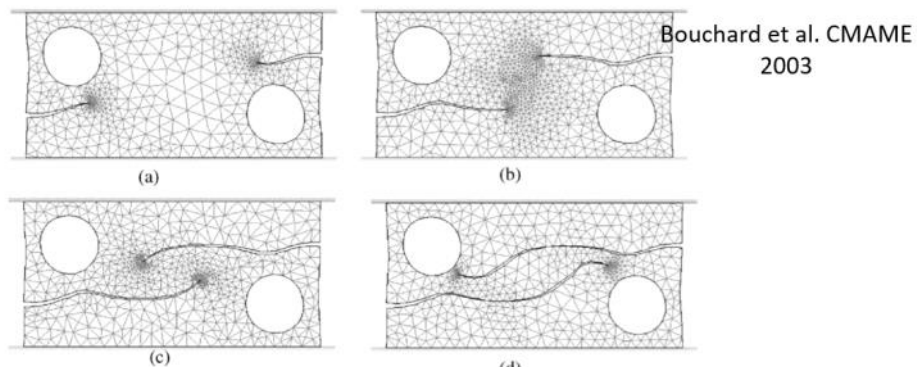


new models  
don't have  
the limitations of  
original models

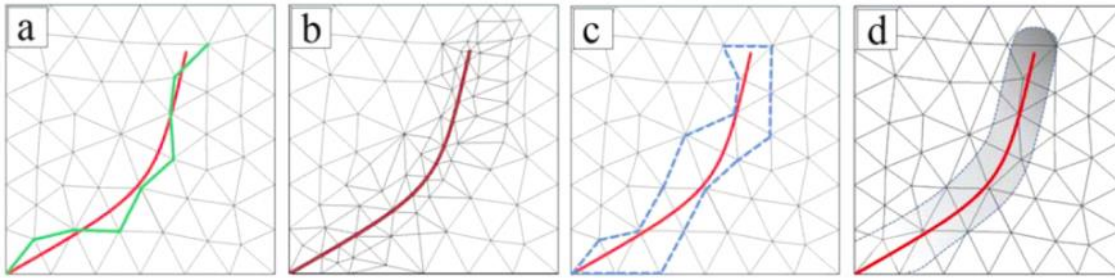
Advantages and challenges of different models

# What's wrong with FEM for crack problems

- Element edges must conform to the crack geometry: make such a mesh is time-consuming, especially for 3D problems.
- Remeshing as crack advances: difficult. Example:



# Capturing/tracking cracks



Fixed mesh

Crack tracking

XFEM enriched  
elements

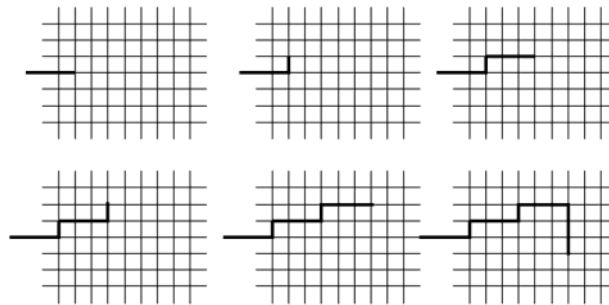
Crack/void capturing by  
bulk damage models

Brief overview in  
the next section

Brief overview in  
continuum damage models

## Fixed meshes

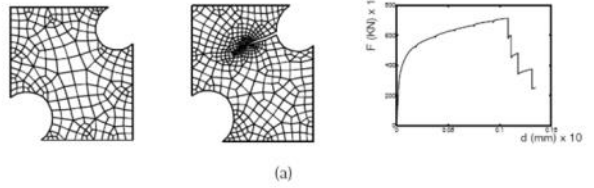
- Nodal release method (typically done on fixed meshes)
  - Crack advances one element edge at a time by releasing FEM nodes
  - Crack path is restricted by discrete geometry



- Also for cohesive elements they can be used for both extrinsic and intrinsic schemes. For intrinsic ones, cohesive surfaces between all elements induces an artificial compliance (will be explained later)

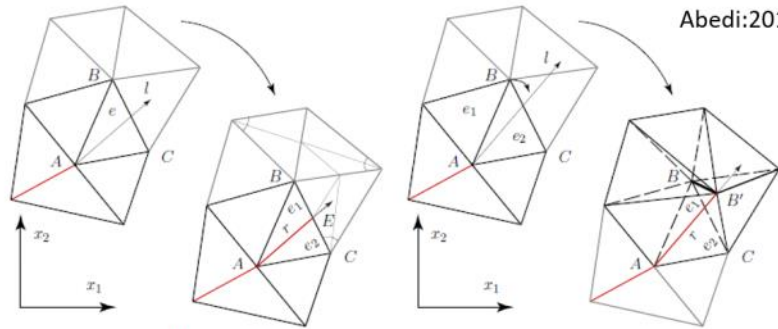
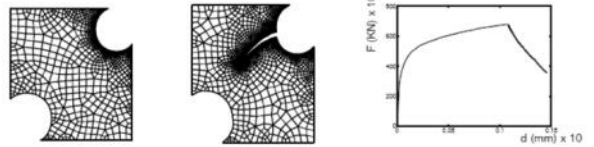
# Adaptive meshes

- Adaptive operations align element boundaries with crack direction



## Element splitting:

Smoother crack path by element splitting: cracks split through and propagate between newly generated elements



Abedi:2010

Cracks generated by **refinement** options

Element edges move to **desired** direction