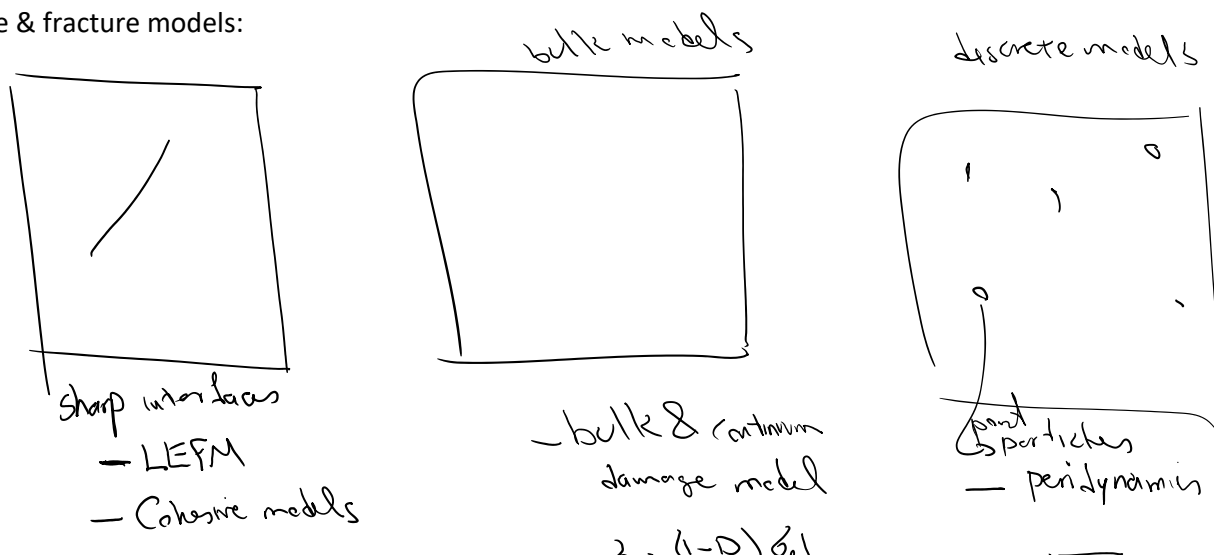


Summary:

Failure & fracture models:



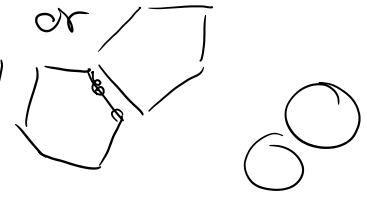
- Cohesive models

Challenge modelling the geometry

damage model

$D = (1-D) \delta_{el}$
- phase field regularized LEM

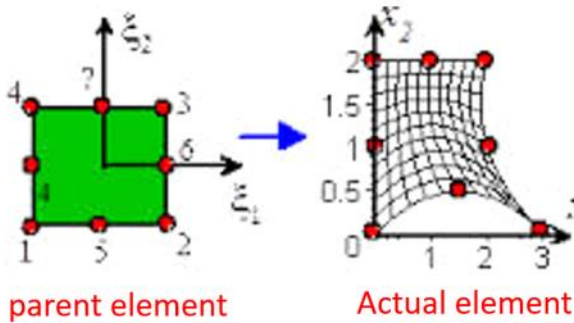
- peridynamics



Discrete Element method (DEM) geomechanics

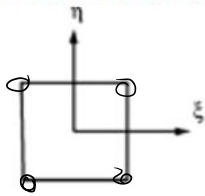
focus of this fracture course

Isoparametric Elements



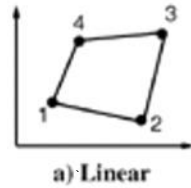
- Geometry is mapped from a parent element to the actual element
- The same interpolation is used for geometry mapping and FEM solution (in the figure 2nd order shape functions are used for solution and geometry)
- Geometry map and solution are expressed in terms of ξ

Order of element

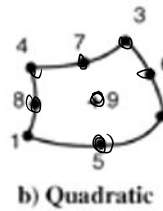


Interpolation space
(Same number of nodes based on the order)

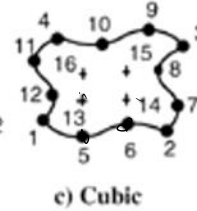
$p=1$



$p=2$

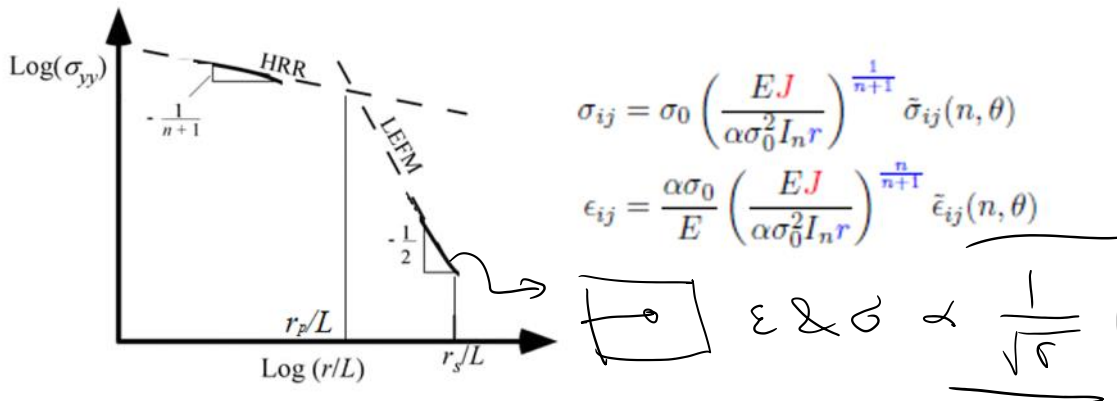


$p=3$

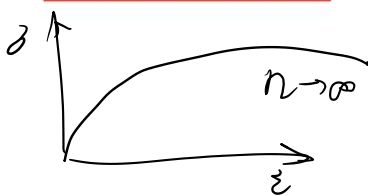


Physical space

Singular crack tip solutions



- **NLFM (PFM)**: For HRR solution stress $\frac{1}{r^{1/(n+1)}}$ and strain $\left(\frac{1}{r}\right)^{n/(n+1)}$ are still singular \Rightarrow
- for elastic-perfectly plastic ($n \rightarrow \infty$) stress is bounded and strain is $\left(\frac{1}{r}\right)$ singular



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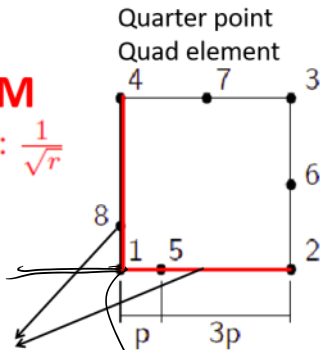
$\epsilon \rightarrow$ elastic perfectly plastic

Isoparametric singular elements



• **LEFM**

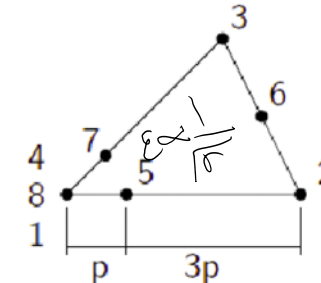
$\epsilon, \sigma \propto \frac{1}{\sqrt{r}}$



singularity form $\frac{1}{\sqrt{r}}$ only along these lines NOT recommended

Handwritten note: 'Want this point to be a crack tip'

Quarter point collapsed Quad element

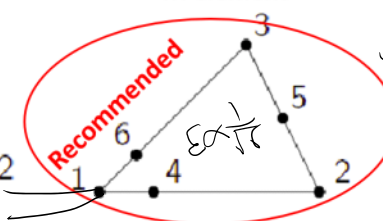


Improvement:
- $\frac{1}{\sqrt{r}}$ from inside all element

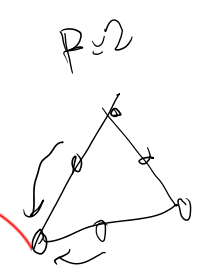
Problem

- Solution inaccuracy and sensitivity when opposite edge 3-6-2 is curved

Quarter point Tri element



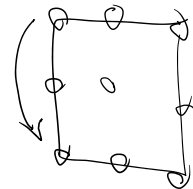
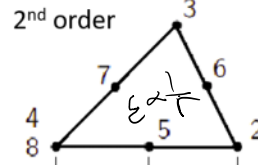
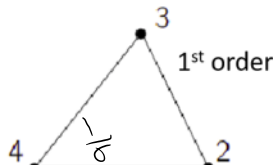
Improvement:
- Better accuracy and less mesh sensitivity



PFM approximating elastic perfectly plastic singularity

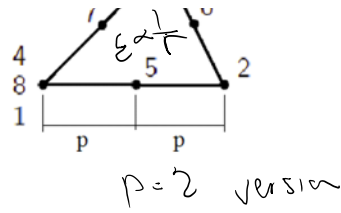
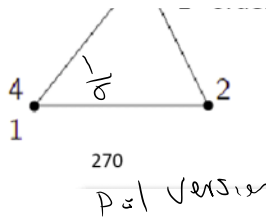
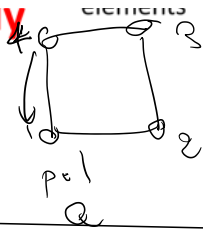
• **Elastic-perfectly plastic**

Collapsed Quad elements



perfectly plastic

$$\epsilon : \frac{1}{r}$$



Why the singularity is generated?

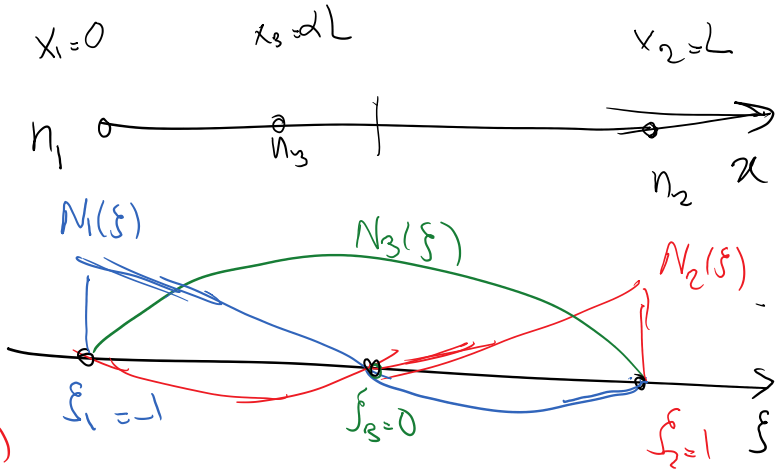
$$N_1(\xi) = \frac{(\xi - \xi_2)(\xi - \xi_3)}{(\xi_1 - \xi_2)(\xi_1 - \xi_3)}$$

$$= \frac{(\xi - 1)(\xi - 0)}{(-1 - 1)(-1 - 0)} = \frac{\xi(\xi - 1)}{2}$$

$$N_2(\xi) = \frac{(\xi - \xi_1)(\xi - \xi_3)}{(\xi_2 - \xi_1)(\xi_2 - \xi_3)}$$

$$= \frac{(\xi - (-1))(\xi - 0)}{(1 - (-1))(1 - 0)} = \frac{\xi(\xi + 1)}{2}$$

$$N_3(\xi) = \frac{(\xi - \xi_1)(\xi - \xi_2)}{(\xi_3 - \xi_1)(\xi_3 - \xi_2)} = \frac{(\xi - (-1))(\xi - 1)}{(0 - (-1))(0 - 1)} = 1 - \xi^2$$



$$u(\xi) = N_1(\xi) u_1 + N_2(\xi) u_2 + N_3(\xi) u_3$$

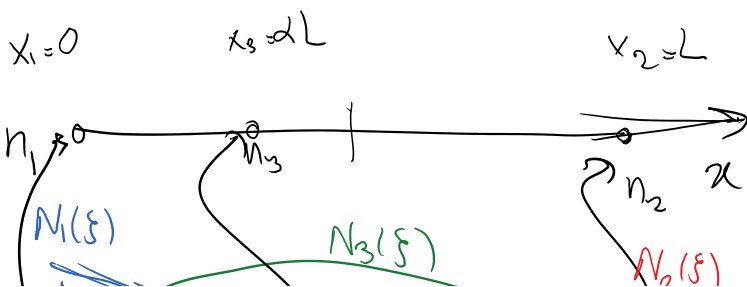
u_1, u_2, u_3 are nodal displacement values

solution



$$u(\xi = -1) = \alpha N_1(-1) + u_2 N_2(-1) + u_3 N_3(-1) = 1 \cdot u_1$$

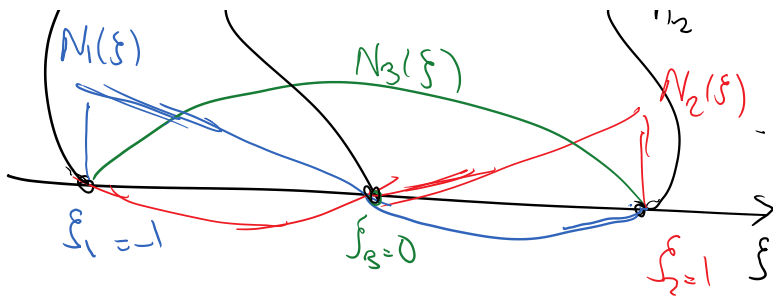
is parametric
 geometry has the same level of approximation as solution



$$x(\xi) = \alpha_1 N_1(\xi) + \alpha_2 N_2(\xi) + \alpha_3 N_3(\xi)$$

obviously $x(\xi_1) = \alpha_1$
 $\alpha_1 = 0$ $\alpha_2 = L$ $\alpha_3 = \alpha L$

$$x(\xi) = L \frac{\xi(\xi + 1)}{2} + (\alpha L)(1 - \xi^2)$$



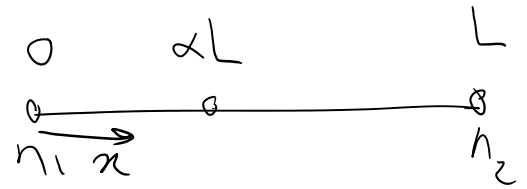
$$x_1 = 0 \quad x_2 = L \quad x_3 = \alpha L$$

$$x(\xi) = L \frac{\xi(\xi+1)}{2} + (\alpha L)(1-\xi^2)$$

$$x(\xi) = L \left(\alpha + \frac{\xi}{2} + \xi^2 \left(\frac{1}{2} - \alpha \right) \right) \quad (*)$$

Recall $u(\xi) = u_1 N_1(\xi) + u_2 N_2(\xi) + u_3 N_3(\xi)$

I want to achieve the desired $\frac{1}{\sqrt{x}}$ singularity around point \downarrow for strain



$$\epsilon = \frac{du}{dx} = \frac{du}{d\xi} \frac{d\xi}{dx}$$

$\epsilon(x \rightarrow 0) \rightarrow \infty$ requires $\frac{dx}{d\xi} \rightarrow 0$ at $\xi = -1$

$$(*) \rightarrow \frac{dx}{d\xi} = L \left(\frac{1}{2} + 2\xi \left(\frac{1}{2} - \alpha \right) \right)$$

$$\text{we want } \frac{dx}{d\xi}(\xi = -1) = 0 \rightarrow \frac{1}{2} - 2 \left(\frac{1}{2} - \alpha \right) = 0$$

$$\alpha = \frac{1}{4}$$

plug

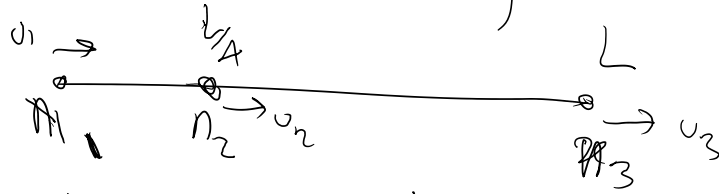
$$\alpha = \frac{1}{4}$$

in (*)

$$P_{\alpha = \frac{1}{4}}: x = \frac{L}{4} (\xi+1)^2 \rightarrow \xi = 2\sqrt{\frac{x}{L}} - 1$$

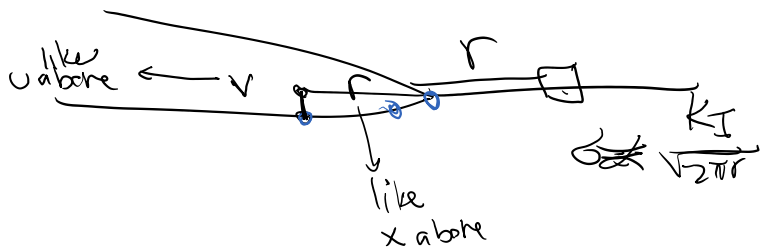
$$u = u_1 \underbrace{\frac{\xi(\xi-1)}{2}}_{N_1(\xi)} + u_2 \underbrace{\frac{\xi(\xi+1)}{2}}_{N_2(\xi)} + u_3 \underbrace{(1-\xi^2)}_{N_3(\xi)}$$

$$u = u_1 + \sqrt{\frac{x}{L}} (-3u_1 - u_2 + 4u_3) + 2\frac{x}{L} (u_1 + u_2 - 2u_3)$$

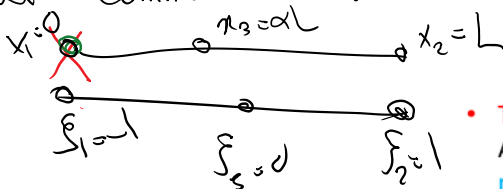


$$E(x) = \frac{du}{dx} = \frac{1}{\sqrt{xL}} \left(-\frac{3}{2}u_1 - \frac{1}{2}u_2 + 2u_3 \right) + \frac{2}{L} (u_1 + u_2 - 2u_3)$$

LEFM

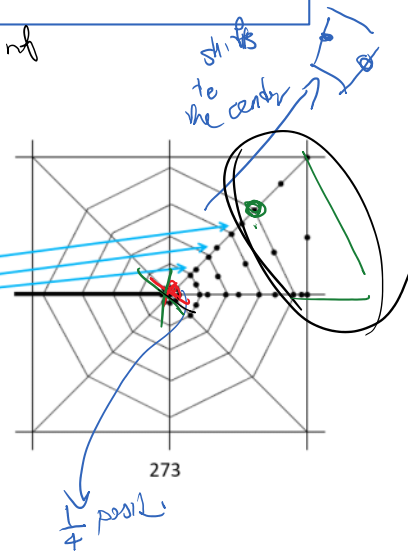


Additional comments about the localis of this point



- **Transition elements:** According to this analysis mid nodes of next layers move to 1/2 point from 1/4 point

Lynn and Ingraffea 1977)



$$\frac{d\alpha}{d\zeta} = L \left(\frac{1}{2} + \zeta(1-2\alpha) \right)$$

X: singularity at the element edge $\rightarrow \alpha = \frac{1}{4}$

as $\zeta \rightarrow \infty$ for the point singularity

$$\frac{1}{2} + \zeta(1-2\alpha) = 0 \rightarrow$$

$$\alpha = \frac{1}{2} + \frac{1}{4\zeta}$$

$$\zeta \rightarrow \infty \quad \alpha \rightarrow \frac{1}{2}$$

As we'll see, even with interior meshes we can still get decent solutions for K, J, etc as long as the right method is used.

6.1.3 Extraction of K (SIF) and G = J

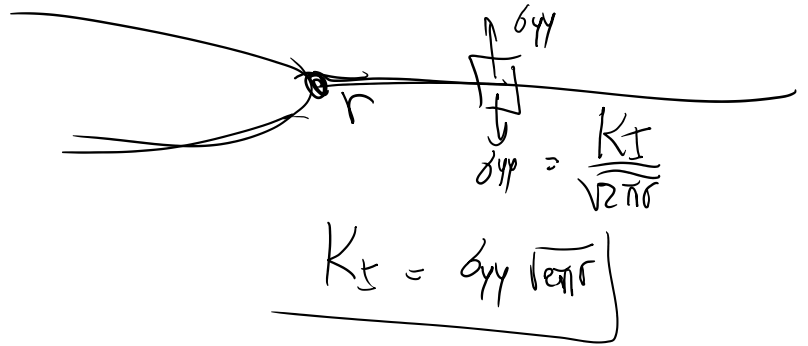
1 $u_y(v)$



1. u_y (v)

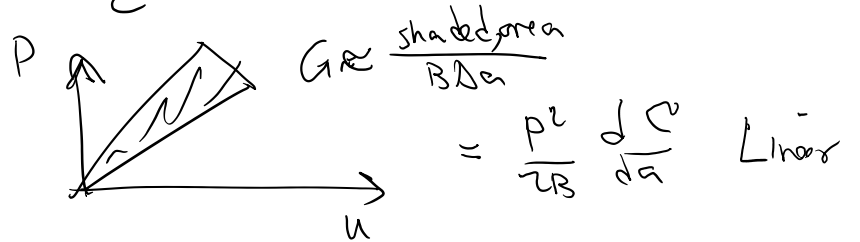
$2u_y \sqrt{r} K \quad \frac{u_y}{r} \propto K$

2. δy



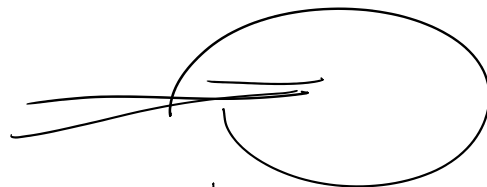
3. K calculated from G (or $\bar{\sigma}$)

mode I $G = \frac{K_I^2}{E'} \rightarrow K_I = \sqrt{G E'}$



$G = - \frac{d\Pi}{B da}$

$G = J$



we'll discuss how these will be used computationally

1. K from local fields

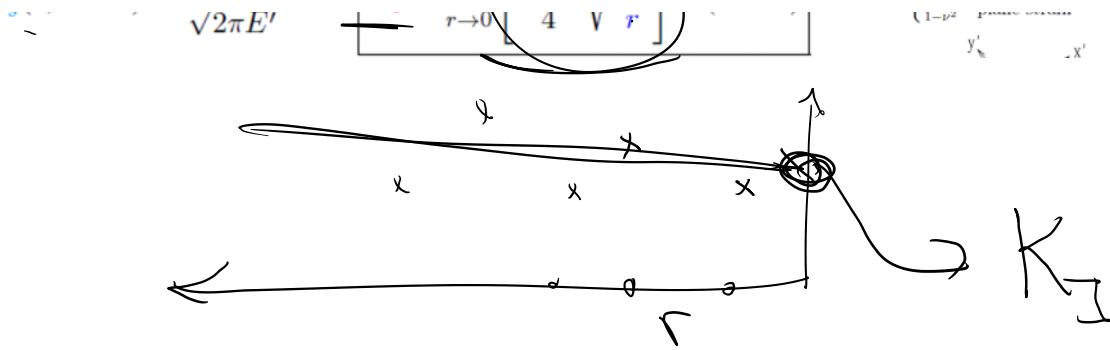
1. Displacement

$u_y(r, \theta = \pi) = \frac{4K_I \sqrt{r}}{\sqrt{2\pi E'}}$

$\Rightarrow K_I = \lim_{r \rightarrow 0} \left[\frac{E' u_y}{4} \sqrt{\frac{2\pi}{r}} \right] (\theta = \pi)$

$E' = \begin{cases} E & \text{plane stress} \\ \frac{E}{1-\nu^2} & \text{plane strain} \end{cases}$

$y'_x \quad -x'$



We'll do a best fit to $K_I(r)$ and evaluate the function at $r = 0$

or alternatively from the first quarter point element:

LEFM

$$v = K_I \frac{\kappa + 1}{2G} \sqrt{\frac{r}{2\pi}}$$

$$u' = \bar{u}'_A + (-3\bar{u}'_A + 4\bar{u}'_B - \bar{u}'_C) \sqrt{\frac{r}{L}} + (2\bar{u}'_A + 2\bar{u}'_C - 4\bar{u}'_B) \frac{r}{L}$$

$$v' = \bar{v}'_A + (-3\bar{v}'_A + 4\bar{v}'_B - \bar{v}'_C) \sqrt{\frac{r}{L}} + (2\bar{v}'_A + 2\bar{v}'_C - 4\bar{v}'_B) \frac{r}{L}$$

$$K_I = \frac{2G}{\kappa + 1} \sqrt{\frac{2\pi}{L}} (-3\bar{v}'_A + 4\bar{v}'_B - \bar{v}'_C)$$

Recall for 1D

$$u = u_1 + \sqrt{\frac{x}{L}} (-3u_1 - u_2 + 4u_3) + \frac{2x}{L} (u_1 + u_2 - 2u_3)$$

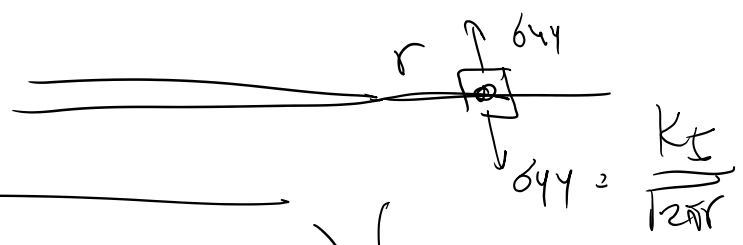


$$\begin{Bmatrix} K_I \\ K_{II} \end{Bmatrix} = \frac{1}{2} \frac{2G}{\kappa + 1} \sqrt{\frac{2\pi}{L}} \begin{bmatrix} 0 & 1 \\ 1 & 0 \end{bmatrix} \begin{bmatrix} -3\bar{u}'_A + 4(\bar{u}'_B - \bar{u}'_D) - (\bar{u}'_C - \bar{u}'_E) \\ -3\bar{v}'_A + 4(\bar{v}'_B - \bar{v}'_D) - (\bar{v}'_C - \bar{v}'_E) \end{bmatrix}$$

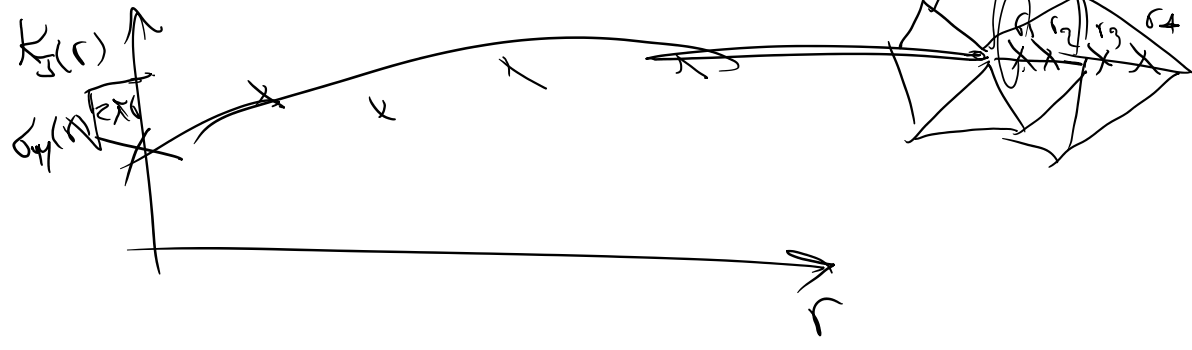
275

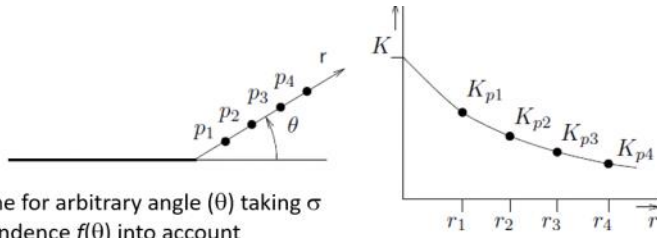
Mixed mode generalization:

2. Stress:



$$K_I = \lim_{r \rightarrow 0} (\sigma_{yy} \sqrt{2\pi r})$$





or can be done for arbitrary angle (θ) taking σ angular dependence $f(\theta)$ into account

Reasons why the stress approach is worse:

1.

$$\sigma \propto \frac{1}{\sqrt{r}} \rightarrow \infty$$

$$u \propto \sqrt{r} \rightarrow 0$$

Even with quarter point elements & spider web mesh, it's difficult to capture the tendency of the stress field to infinity

2.

enough exact FEM with smooth element order

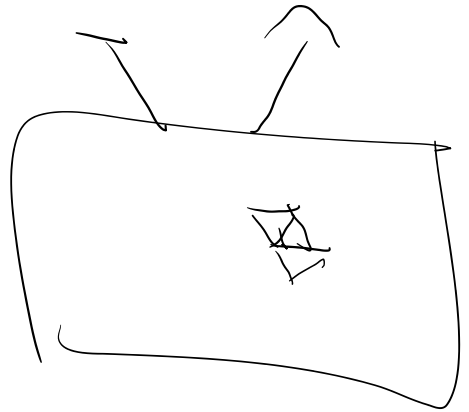
$$\|u^{FEM} - u^{exact}\| = Ch^{p+1}$$

$$\epsilon = \nabla u \quad \downarrow \text{derivative}$$

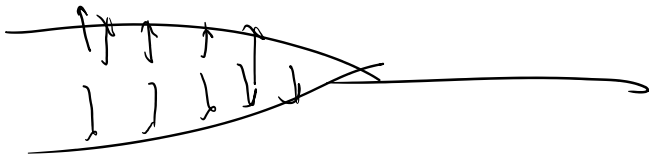
$$\sigma = C \nabla u$$

$$\|\sigma^{FEM} - \sigma^{exact}\| = Ch^p$$

stress solutions are \downarrow power less accurate



3. Stress approach is specifically poor when the crack surfaces are loaded (pressure vessel, H. Fract)



2. K from energy approaches

1. Elementary crack advance (two FEM solutions for a and $a + \Delta a$)
2. Virtual Crack Extension: Stiffness derivative approach
3. J-integral based approaches (next section)

After obtaining G (or $J=G$ for LEFM) K can be obtained from

$$K_I^2 = E' G \quad E' = \begin{cases} E & \text{plane stress} \\ \frac{E}{1-\nu^2} & \text{plane strain} \end{cases}$$